

**THE IMPACT OF LANDSLIDES ON SEDIMENT YIELD,
SOUTH WESTLAND, NEW ZEALAND**

by

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B.Sc. University of Wisconsin – Eau Claire, 2006

THESIS SUBMITTED IN PARTIAL FULFILLMENT OF
THE REQUIREMENTS FOR THE DEGREE OF
MASTER OF SCIENCE

In the
Department of Earth Sciences
Faculty of Science

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SIMON FRASER UNIVERSITY

Summer 2012

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ABSTRACT

Sediment yields in the Southern Alps of New Zealand are among the highest in the world due to high annual precipitation, rapid uplift, weak bedrock and episodic earthquakes. Two neighbouring watersheds, those of the Poerua and Waitangitaona rivers, were studied to determine the impact landslides have on sediment yield. Both watersheds have been recently disturbed by landslides, 1) a large rock avalanche from Mt. Adams in the Poerua River watershed and 2) a failing slope known as the “Gaunt Creek slip” in the Waitangitaona watershed. I conducted a ground penetrating radar survey of the lower Waitangitaona River valley and a dGPS topographic survey of the lower Poerua River valley to determine average sediment yields on different timescales. The estimated sediment yields, which are among the highest in the world, are controlled by differences in the time, size, and character of the landslides that have perturbed the fluvial system.

Keywords: landslide; sediment yield; geomorphology; ground-penetrating radar; dGPS survey; South Westland, New Zealand

To everyone who asked me, “When are you going to finish?”.

“Soon”

ACKNOWLEDGEMENTS

Completion of this project would not have been possible without the generous contributions of number of people. First, I give my highest gratitude to my senior supervisor, Dr. John Clague, for his support, guidance, financial support and most importantly his patience throughout this endeavor. Thank you for the opportunity. It was an unbelievable privilege to study under you and learn from your knowledge and experience.

I would also like to thank my committee members; Dr. Shahin Dashtgard, Dr. Tim Davies, Dr. Oliver Korup and Dr. Mauri McSaveney. Their support, comments and suggestions were paramount. I would also like to thank the external examiner, Dr. Brian Menounos, for his constructive comments during the revision process.

Thanks to the faculty and staff of the Earth Sciences Department of SFU, particularly the graduate secretary Glenda Pauls for her help with graduate student issues, and the department technical staff Matt Plotnikoff and Rodney Arnold. Thank you also to my lab mate and fellow Natural Hazards Research Group members for their support, insight, and friendship, including: Nick Roberts, Denny Capps, Courtenay Brown, Dan Shugar, Marit Heideman and last but, not least, Michelle Hanson including her assistance in the field.

Finally, I thank my family with all my heart for their unconditional love, support, and encouragement. Mom and Dad, Kristin, thanks and this is for you.

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CHAPTER 1: INTRODUCTION

Landslides are common in mountainous areas throughout the world. They are an important agent of denudation (Burbank et al. 1996) and a major source of sediment to streams. Landslides occur frequently in the Southern Alps of New Zealand because of the high relief, steep slopes, weak bedrock, frequent earthquakes, and high precipitation.

In some instances landslides block streams, creating lakes upvalley of the debris dams. Unlike engineered dams, landslide barriers consist of unsorted and unconsolidated material that is susceptible to failure by piping, collapse, and incision by overflowing waters (Costa 1985; Costa and Schuster 1988). Failure of a landslide dam may result in catastrophic flooding, downstream aggradation and avulsion, and secondary landslides. Downstream sedimentation commonly increases the possibility of subsequent flooding (Hancox et al. 2005; Schuster 2006).

The sediment “pulse” that results from breaching of a landslide dam can reduce the average grain size of the channel through aggradation (Sutherland et al. 2002), increase channel width (James 1991), promote braiding of the channel, and bury riparian forest (Sutherland et al. 2002). Subsequently, with no further instability within the watershed and on timescales of years to tens of years, the aggraded channel is incised and the channel sediments becomes coarser.

A recent example of landslide damming followed by dam failure in the Southern Alps is the Mt. Adams event in the Poerua River watershed near Hari Hari, South Westland (Fig. 1.1) On 6 October 1999, a large rock avalanche from Mt. Adams impounded Poerua

River, creating a lake. The lake overtopped the dam one day later and failed in another five days, causing aggradation, avulsion, and flooding downstream (Hancox et al. 2005).

Because most landslide dams are unstable, the hazards from outburst floods have been the main focus of recent research. On the geologic time scale, landslide dams are ephemeral features (Korup 2002). Schuster (1993) reported that most landslide dams fail within one year of formation. However, Clague and Evans (1994) documented landslide dams in the Canadian Cordillera that are hundreds to thousands of years old. In addition, the largest landslide dam on Earth was emplaced by the Usoi landslide in Tajikistan in 1911. The dam is 500-700 m high (Alford et al. 2000) and has been well documented since its formation.

The subsequent, secondary effects of outburst floods have not been well documented and are rare. Bathurst and Ashiq (1998) studied bedload transport 13 years after a dam-break flood event, and recently Morche and Schmidt (2011) documented a decade-long study of pre- and post-dam-break sediment transport in the Bavarian Alps.

Several factors determine how a landslide affects a stream. The most important factors are the size of the landslide, the character and size of the dam, and the fluvial response over time. This thesis focuses on the last of these three factors.

The main objectives of this thesis are to: 1) present new data on the geomorphic imprint of the Mt. Adams landslide on Poerua River between 1999 and 2008 (Chapter 2); and 2) quantify sediment yield in the Waitangitaona River watershed (Fig. 1), southwest of Poerua River (Chapter 3). In Chapter 2, I describe the results of a differential GPS survey that extends earlier surveys between 1999 and 2005. I present changes in channel geomorphology and attempt to quantify sediment discharge and sediment yield. I

compare my findings for the period 2005-2008 to changes in the river between 1999 and 2005. In Chapter 3, I describe the results of a ground-penetrating radar (GPR) survey of the lower Waitangitaona River valley carried out to demarcate the floodplain surface prior to an avulsion event in 1967. I estimate the total amount of sediment deposited since the avulsion and derive an estimate of average sediment yield over the 40-year period. Finally, I compare this value to other published estimates of sediment yield in the Southern Alps. Chapter 4 discusses the implications of the two studies for fluvial systems in South Westland.

1.1 Study Area

The study area is located on the coastal plain between the Southern Alps and Tasman Sea in South Westland, New Zealand (Fig. 1.1). The coastal plain is 10-15 km wide, extends from sea level to ca. 200 m asl, and has low to moderate relief. I conducted research on the alluvial plains of Poerua and Waitangitaona rivers, west of the Southern Alps.

Directly east of the study area, the Southern Alps rise abruptly to almost 4000 m asl on the east side of the Alpine Fault. The mountain range is a product of the convergence of the Pacific and Australian plates and is characterized by steep slopes, deep valleys with modal slopes of 38–40°, and boulder-choked streams (Korup 2005b; Wells and Goff 2005).

Precipitation is common throughout the year and can reach 14,000 mm a⁻¹ (Henderson and Thompson 1999); average rainfall in the front ranges of the Southern

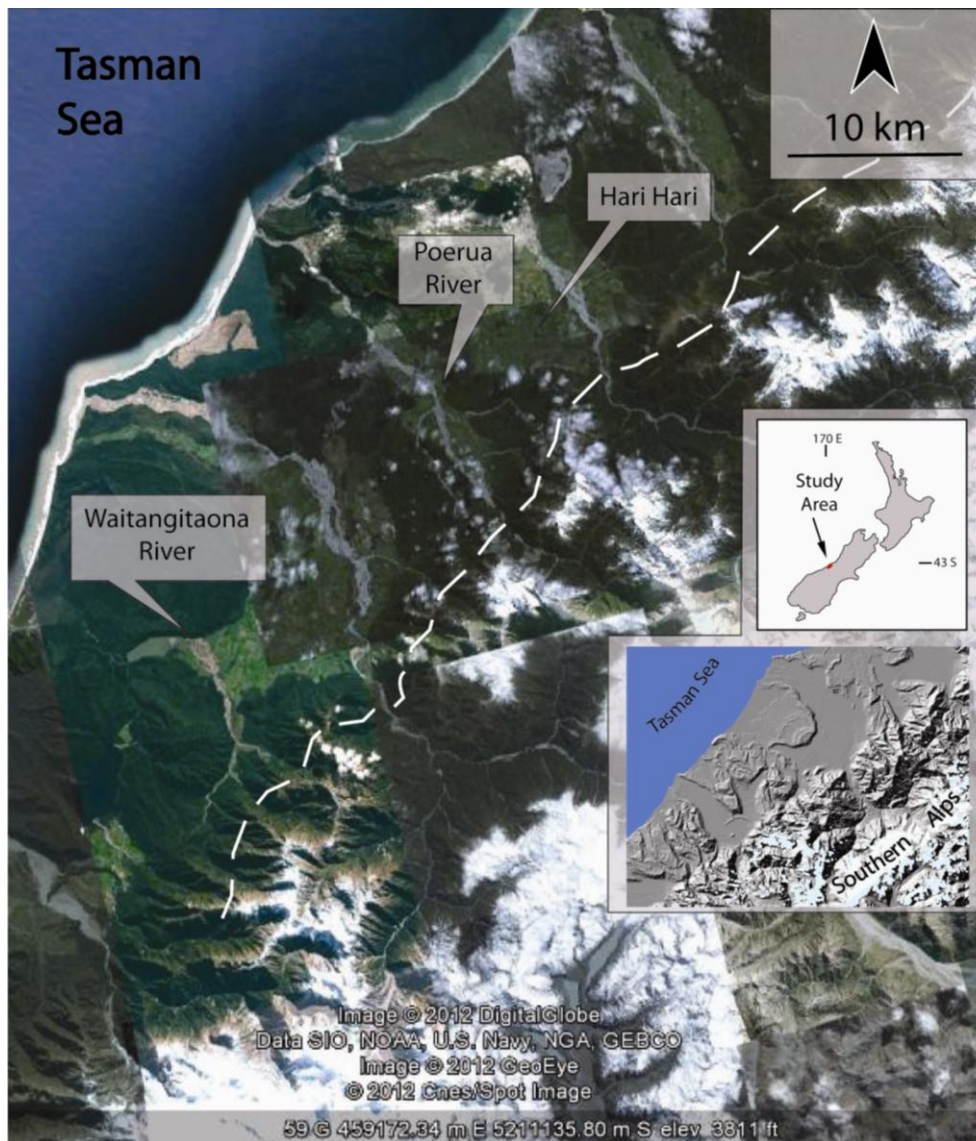


Figure 1.1. The study area, showing the locations of Poerua and Waitangitaona rivers (Google Earth 2011). The Alpine Fault is delineated by the white dashed line. Inset shaded-relief map shows the topography of the study area.

Alps is $11,000 \text{ mm a}^{-1}$ (Griffiths and McSaveney 1983). The combination of steep slopes, weak rocks, and high rainfall results in high erosion and large sediment fluxes.

Catchments in the Southern Alps have some of the highest sediment yields in the world (Davies and McSaveney 2001).

1.1.1 Geology

The South Westland coastal foreland lies west of the Alpine Fault and is underlain by early Ordovician greywacke capped by a thick sequence of Pleistocene glaciofluvial deposits and till (Korup 2005a; Cox and Barrel 2007). The Southern Alps have formed mainly during the past 2 million years and are composed of deformed and metamorphosed, Carboniferous to Cretaceous greywacke; rocks within 8 km to the east of the Alpine Fault are schist (Fig. 1.2; Cox and Findlay 1995; Cox and Barrel 2007).

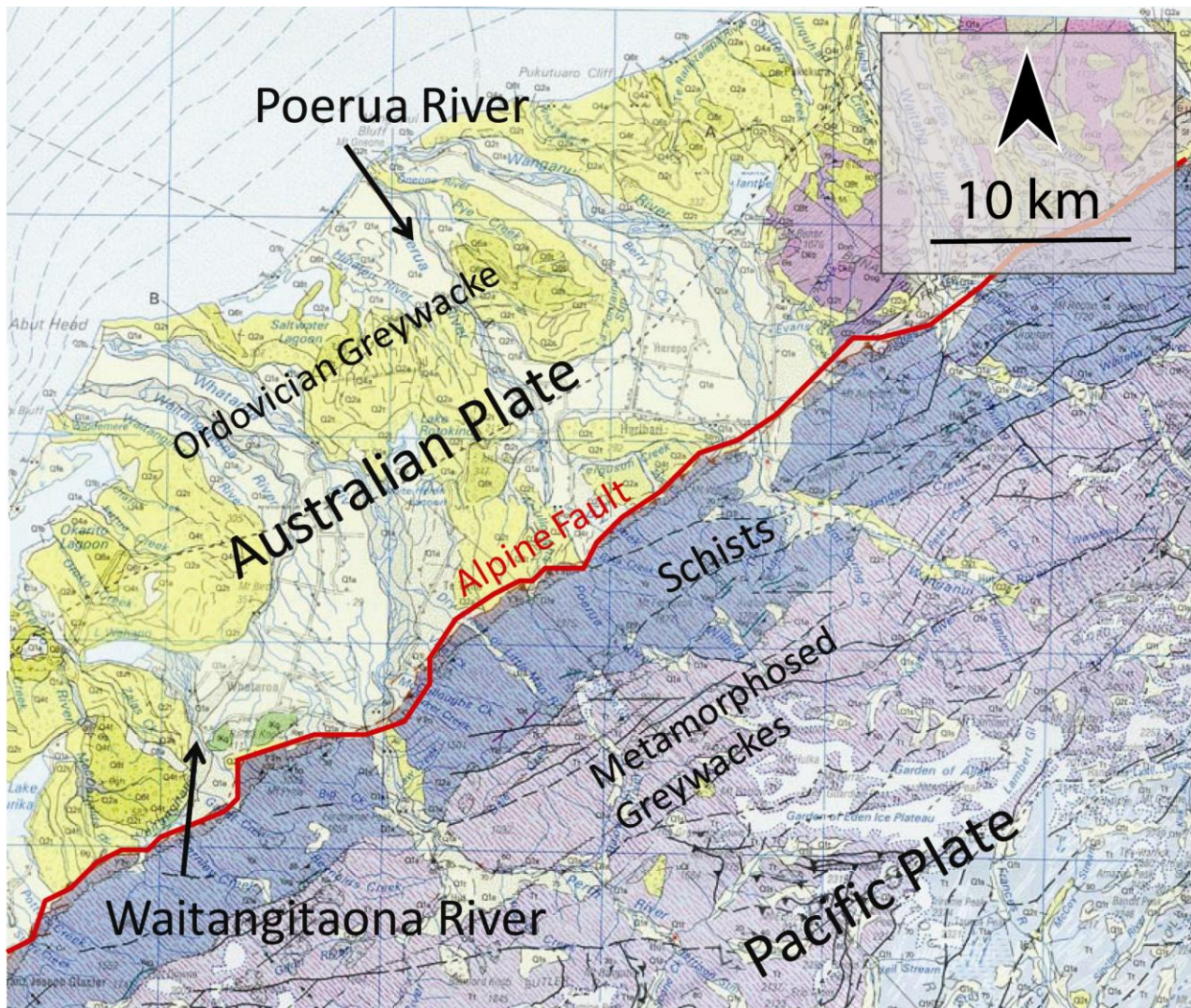


Figure 1.2. Geologic map of the Southern Alps and the Tasman Sea coastal lowland (Cox and Barrel 2007). The red line marks the Alpine Fault.

Prominent morainal ridges of the last glaciation have been assigned to the Okarito and Moana formations, which were deposited during the Kumara 2₂ (~18,000 to 22,000 ¹⁴C years BP) and Kumara 3₂ (~14,000 to 15,000 ¹⁴C years BP) stages, respectively, of the last (Otira) glaciation (Suggate 1990). The tills consist mainly of sandy, gravelly diamicton with clasts of granite, schist, and greywacke (Denton and Hendy 1994). Low areas between the morainal ridges and plateaus are outwash plains and Holocene floodplains (Warren 1967).

1.1.2 *Tectonics*

The Australian and Pacific plates are moving against each other at a rate of 37 ± 2 mm a⁻¹; 75% of this motion is taken up by strike-slip and dip-slip displacements along the Alpine Fault (Fig. 1.3) (Norris and Cooper. 2000).



Figure 1.3. Oblique aerial photograph of the Alpine Fault; view north from the study area to the southern tip of the North Island of New Zealand (McSaveney 2007).

The hanging wall is composed of a sequence of pumpellyite-actinolite schists (Cox and Findlay 1995). Average long-term displacements along the fault at Waikukupa River can be resolved into dip-slip (thrust) component of $8 \pm 3 \text{ mm a}^{-1}$ and a strike-slip component of $27 \pm 5 \text{ mm a}^{-1}$ (Norris and Cooper 2000). Average uplift rates in the Southern Alps locally reach 10 mm a^{-1} (Norris and Cooper 2007).

1.1.3 Alpine Fault Activity

No large earthquakes have occurred on the Alpine Fault during the past 200 years (Sutherland et al. 2006). There is geologic evidence, however, for five great ($M_w \sim 8$) earthquakes since about AD 900 (Larsen et al. 2005). Their approximate ages have been determined from radiocarbon ages on plant fossils in sediments associated with the earthquakes and by dendrochronology. The ages of the earthquakes are AD 940 ± 50 , 1220 ± 50 , 1425 ± 15 , 1620 ± 10 , and 1717 (Wells et al. 1998, 1999; Yetton 1998; Yetton et al. 1998; Norris et al. 2001; Wells and Goff 2005). According to Yetton (2000), the average recurrence time of an earthquake producing an 8-m displacement on the fault is ~ 300 years. The 1717 AD event produced a horizontal offset up to 8.5 m and uplift of about 3 m (Yetton 1998).

CHAPTER 2: WAITANGITAONA RIVER WATERSHED SEDIMENT YIELD AND DENUDATION RATES

2.1 Introduction

Sediment yield in mountain catchments is controlled by many factors, some of which vary in time and differ in space (Warrick and Mertes 2009). The main controlling factors are climate, geology, relief, and land use (Meade et al. 1990). High rates of sediment yield are favoured by high rainfall, weak bedrock, high relief, rapid uplift, frequent large earthquakes, and extensive modification of the landscape by humans (Griffiths 1981; Milliman and Syvitski 1992; Hicks et al. 1996). Most of these conditions exist in the Southern Alps of New Zealand – annual precipitation locally exceeds 14 m; the area is characterized by steep slopes developed in metamorphosed and pervasively deformed greywacke and schist close to the Alpine Fault, the active transform fault separating the Pacific and Australian plates; the maximum relief in the range is 4000 m, and local relief exceeds 2000 m; and mountains are rising at rates up to 10 mm a⁻¹ (Cox and Findlay 1995; Norris and Cooper 1997; Henderson and Thompson 1999).

Contemporary sediment yield in New Zealand on an annual timescale has been estimated from sediment discharge records at hydrometric stations (Griffiths 1979, 1982; Hicks et al. 1996, 2004). There are few estimates, however, of sediment yield on timescales of decades. This study uses estimates of the volume of alluvium deposited by Waitanitaona River, which drains a mid-size watershed in the Southern Alps, since a major avulsion in 1967. From this sediment volume of, I estimate of average sediment

yield over for the Waitangitaona River watershed for the 40-year period. I then compare this value to other estimates of sediment yield in active orogens around the world.

2.2 Study Area

Waitangitaona River drains ~74 km² of the Southern Alps in South Westland, New Zealand. Within the Southern Alps, the river flows in a steep-sided valley developed in fractured and faulted, foliated, biotite schist (Norris and Cooper 2007). The valley and floodplain broaden where Gaunt Creek flows into Waitangitaona River (Fig. 2.1). Over the next 2 km, between Gaunt Creek and the SH (State Highway) 6 bridge, the river is bordered by large Pleistocene moraines and has a floodplain that is 120 to 850 m wide. Below the bridge, the river is confined by engineered levees (“stop banks”). Prior to settlement, the river flowed across a ca. 19 km² alluvial fan that is bordered on the north by the Okarito moraine, on the west by the Zalas Creek moraine and Lake Wahapo, and on the east by Waitangirotto River (Fig. 2.1).

During a flood in 1967, Waitangitaona River abandoned its channel and began to flow to the southwest and into Lake Wahapo (Fig. 2.1; Griffiths and McSaveney 1986; Korup 2004). The event occurred after years of aggradation behind a stop bank built in 1933 (Griffiths and McSaveney, 1986). Following avulsion, 4.1 km² of grazing land was buried by a thickening sheet of gravel and sand, confined between stop banks to the west and east (Fig. 2.1). In 1970, three years after the avulsion, the prograding sediment wedge reached the shore of Lake Wahapo and two fan-toe deltas began to form (Korup 2004). Aggradation during the lead-up to the avulsion event in 1967 has been ascribed

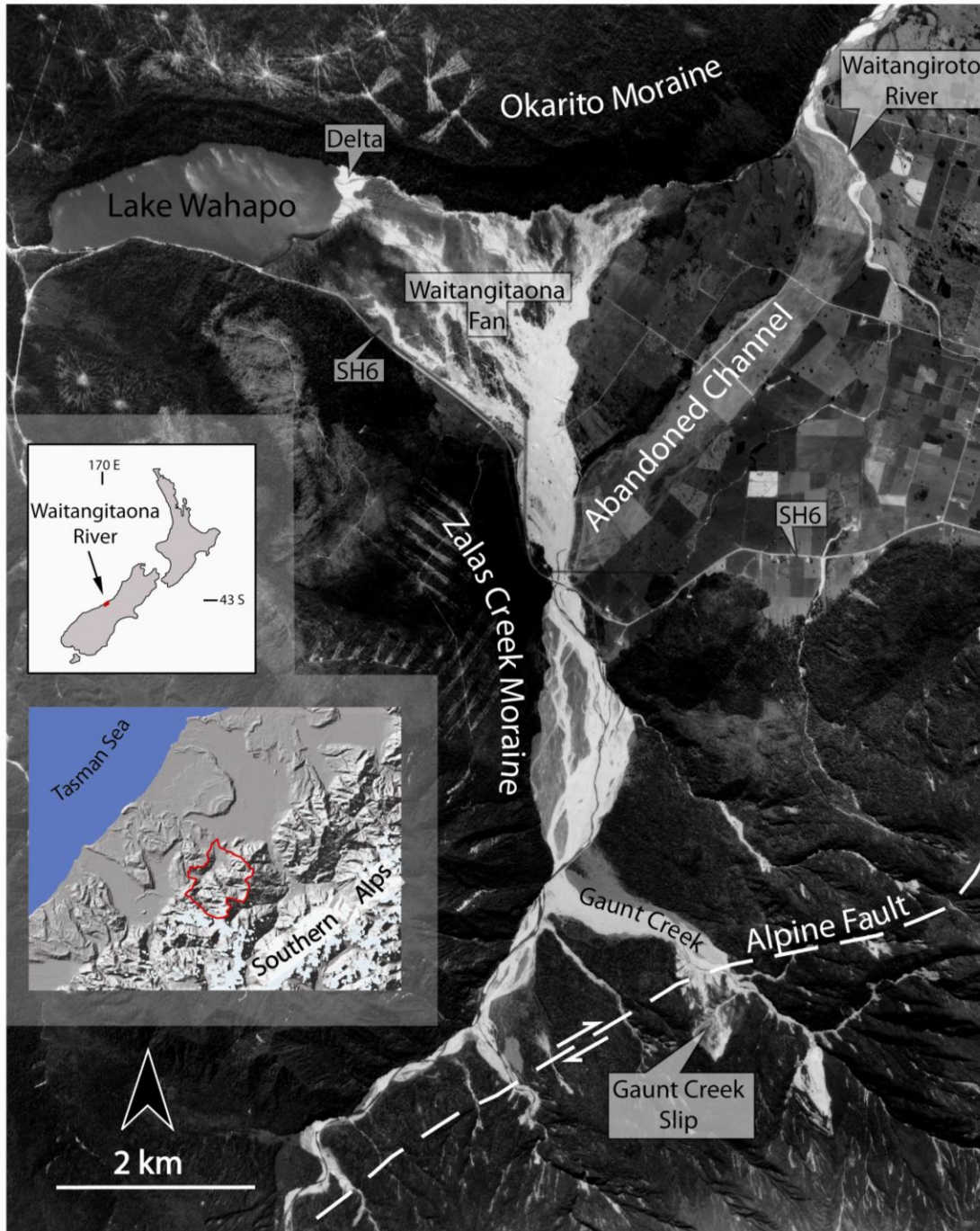


Figure 2.1. Aerial photograph of the study area, showing the site of the 1967 Waitangitaona River avulsion in South Westland, New Zealand. Also shown is Gaunt Creek, the location of the Gaunt Creek slip, and Lake Wahapo. The aerial photograph was taken in 1985 and was provided by Land Information New Zealand. Inset shaded relief map shows the Waitangitaona River watershed (red line). Image modified from Korup (2004).

to the delivery of large amounts of sediment by Gaunt Creek, a tributary of Waitangitaona River. Much of the sediment was derived from a failing slope on the trace of the Alpine Fault that is known informally as the “Gaunt Creek slip” (Fig. 2.1; Griffiths and McSaveney 1986). Davies and Korup (2007) estimated that the Gaunt Creek slip has delivered approximately $1.6 \times 10^5 \text{ m}^3 \text{ a}^{-1}$ of sediment into the Waitangitaona fluvial system since about 1918.

2.3 Methods

I estimated coarse sediment yield from the Waitangitaona River watershed for the period 1967-2008 by determining the volume of sediment deposited following channel avulsion in 1967. The methods that I used include ground-penetrating radar (GPR), multi-temporal aerial photograph interpretation, backhoe trenching, and a differential GPS survey. The GPR survey provided data on the thickness of sediment overlying the pre-avulsion surface and the wedge of post-1967 deltaic sediment in Lake Wahapo. The results of the GPR survey were corroborated by geomorphic interpretation of pre- and post-avulsion aerial photographs and by backhoe trenching.

2.3.1 Ground-Penetrating Radar

Ground-penetrating radar uses electromagnetic (EM) waves to characterize the subsurface structure of sediments and rocks (Baker et al. 2007). Differences in electromagnetic properties of Earth materials affect the paths and travel times of EM waves, revealing structural and textural differences of near-surface materials (Van Overmeeren 1998).

I used a Sensors and Software pulse EKKO 100 system to perform the GPR survey. I conducted tests using two different sets of antennas – 50 and 100 MHz – and several CMP (Common Mid-Point) surveys to determine which antennas were best suited for the study. I also used the CMP surveys to determine the signal velocity for each survey line. The velocities range from 0.07 m n s^{-1} on the Lake Wahapo delta to 0.1 m ns^{-1} on the Waitangitoana fan. The pre-avulsion surface was easily identified using the 100 Mhz antennas, thus all subsequent lines were surveyed with them.

The GPR field setup included 100 MHz antennas, a console, a 12 V battery, and a laptop computer for collecting the data. The GPR antennae were set parallel to each other, 1 m apart, and oriented perpendicular to the survey line (Fig. 2.2). The step size was 0.25 m.

Fifteen transects ranging from 200 to 600 m in length and totalling 4.2 km were surveyed within the modern confined floodplain of Waitangitaona River below the SH6 bridge (Fig. 2.3). Locations of lines were chosen partly on the basis of accessibility and partly to ensure that the area sampled was sufficient to determine spatial variations in the form of the pre-avulsion surface. I correctly assumed that this surface was nearly flat, except along the channels that existed immediately before avulsion, which I was able to identify on pre-1967 photographs. Survey lines on the Lake Wahapo delta, however, were oriented parallel to Waitangitaona River to obtain the maximum amount of information on the thickness and form of the post-1970 prograding deltaic wedge. The maximum depth of energy penetration was 24 m, and full resolution was achieved to a depth of 8 m, which was more than adequate to demarcate the pre-avulsion surface.



Figure 2.2. Ground-penetrating radar field survey of the Waitangitona River floodplain below the SH6 bridge. In the foreground are two 100 kHz antennae. Data are being recorded in the background on a waist-mounted laptop computer. Photograph by Mauri McSaveney, 2008.

The data were of high quality and required little post-processing. I did, however, process them using standard techniques (dewow, subtractor, horizontal average tracing, vertical average tracing, and constant gain) to provide the best visual display (Baker et al. 2007). Processing was done using the Sensors and Software program EKKO View Deluxe (Version 1.0). Interpretation was guided by reference to previous work (Davis and Annan 1989; Jol and Smith 1991). I surveyed topographic profiles along each GPR transect using a Topcon dGPS (differential Global Position System) and referenced elevations to the local datum (NZGS1949). The survey points were collected in real time

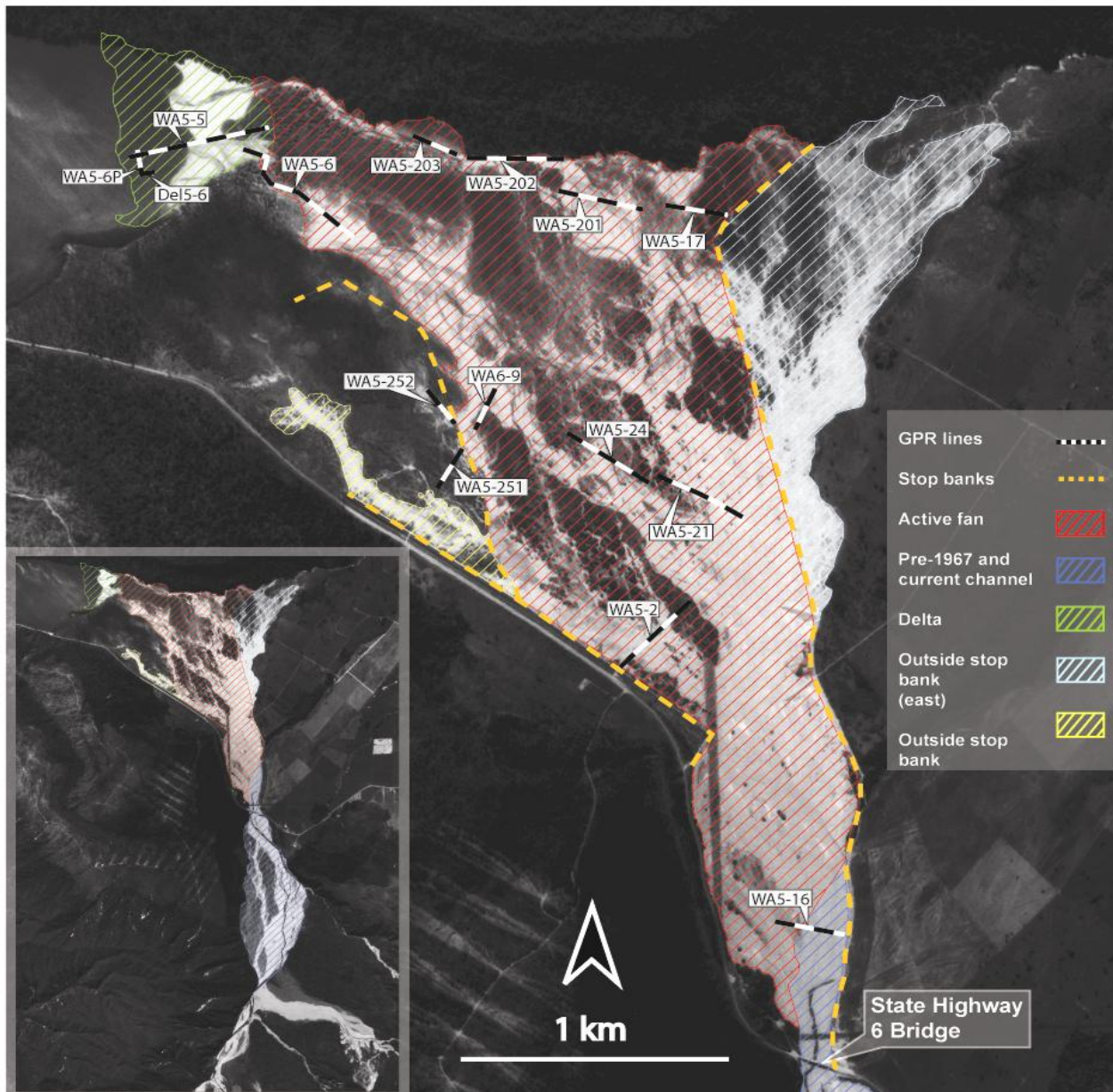


Figure 2.3. 1985 aerial photograph, showing GPR survey lines, stop banks, and the five areas of sediment deposition. Aerial photograph courtesy of Land Information New Zealand.

kinematic (RTK) mode, which provides an accuracy of 10 mm in the horizontal plane and 15 mm vertically. I combined the topographic data and subsurface data in a GIS (Geographic Information System) to estimate sediment volumes. The fan was not buried everywhere by the same thickness of sediment, and not all parts of the fan were surveyed. I thus divided the fan into five areas, each with about the same thickness of

post-1967 sediments to obtain a total sediment volume for the period 1967-2008 (Fig. 2.3).

2.3.2 Map and Aerial Photograph Interpretation

I acquired aerial photographs taken in 1948, 1969, 1973, 1977, 1985, and 1994 from Oliver Korup and examined them to establish the five areas of expected uniform sediment depth. I also used the aerial photographs to determine the progradation rate of Waitangitaona River into Lake Wahapo. I orthorectified the photos in the GIS using ground control points to minimize errors in estimates of progradation rates. Although I chose and located ground control points with caution, minor errors in their positions, unavoidable minor location errors affected the accuracy of the rectification of the photos.

2.3.3 Backhoe Trenching

Backhoe trenches were excavated to a maximum depth of 2.5 m on the fan to confirm reflectors identified in the GPR survey, especially the pre-1967 surface. Because the water table was shallow (average depth = 1 m), the backhoe trench walls collapsed as the trenches were dug, limiting the amount of information that could be acquired. Nevertheless, I was able to identify the pre-1967 surface in one test pit and observed the architecture and sedimentary structures of shallow subsurface sediments.

2.4 Results

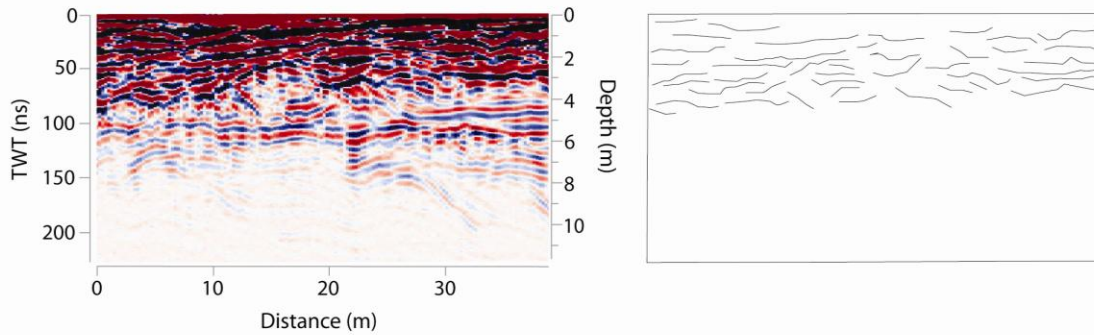
2.4.1 Radar Facies

I identified two radar facies and a continuous subsurface reflector. One radar facies comprises hummocky, subhorizontal, and laterally discontinuous reflectors (Fig.

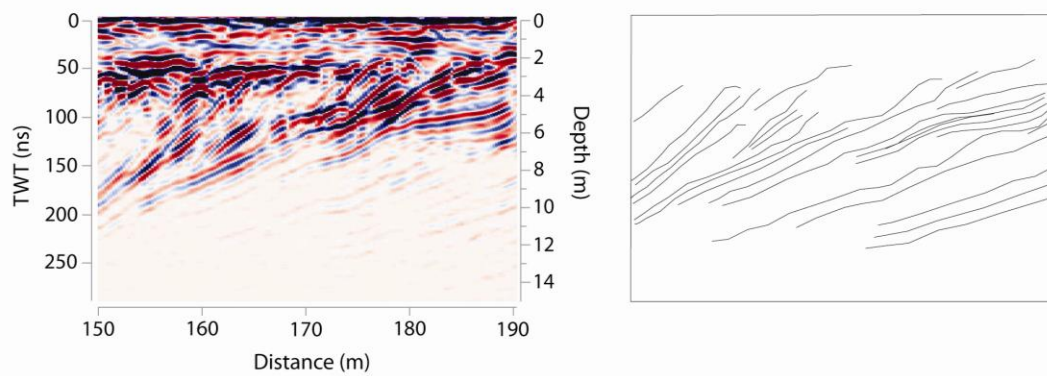
2.4A), which I interpreted to record channel deposits of a braided, gravel-bed stream. This facies dominates most profiles and extends to an average depth of 4 m and a maximum depth of ~7 m at the Lake Wahapo delta. The hummocky nature of the reflectors is likely due to heterogeneity in grain size (Stevens and Robinson 2007) and aggradation (Mumpy et al. 2007). Locally, the reflectors have a concave-up pattern, indicative of small-scale, cut-and-fill channels.

The second radar facies comprises tangential and parallel-dipping reflectors (Fig. 2.4B). It is restricted to the Lake Wahapo delta and adjacent floodplain, and is interpreted to be delta foreset beds. The downstream-dipping foresets are overlain by the braided stream radar facies and terminate in a tangential downlap onto an unseen flat-lying horizon that is below the maximum depth of GPR energy penetration (Mumpy et al. 2007). The unseen horizon is likely the original lake bottom. Survey lines parallel to the flow direction of Waitangitoana River show the foresets with true dip angles, whereas lines perpendicular to stream flow exhibit subhorizontal reflectors along the strike of the beds. True dips range from 12° to 38° and average 24° . The subsurface surface reflector, which is most relevant to this study, is a strong, horizontal, laterally continuous reflector, which I interpret to be the pre-avulsion surface (Fig. 2.4C and 2.5). Aerial photographs show that this surface was stable, vegetated, and perhaps weathered prior to being buried after the avulsion event in 1967. Breaks in the reflector are probably due to erosion of the pre-1967 surface by waters of the avulsed river flowing over the fan.

A. Braided River Facies



B. Foreset Facies



C. Pre-Avulsion Surface

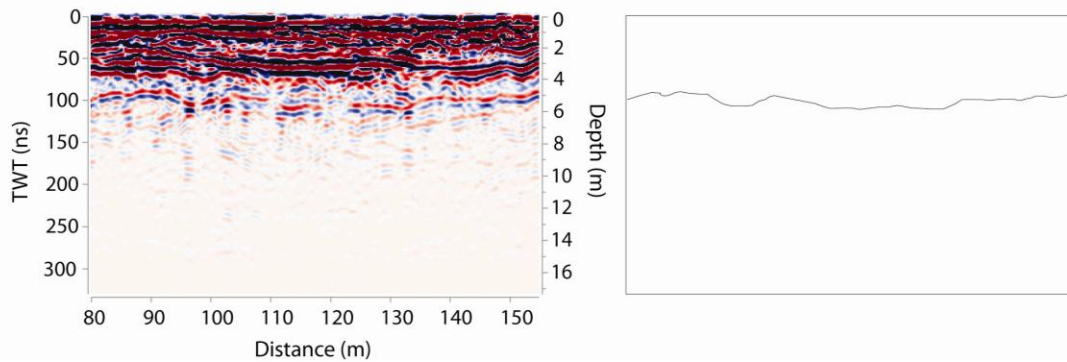


Figure 2.4. Examples of the two radar facies and the subsurface reflector identified in this study. (A) Wavy, subhorizontal, laterally discontinuous reflectors of a braided, gravel-bed stream. (B) Dipping reflectors of foreset facies. (C) Strong, horizontal, laterally continuous reflector of pre-avulsion surface. TWT = two-way travel time.

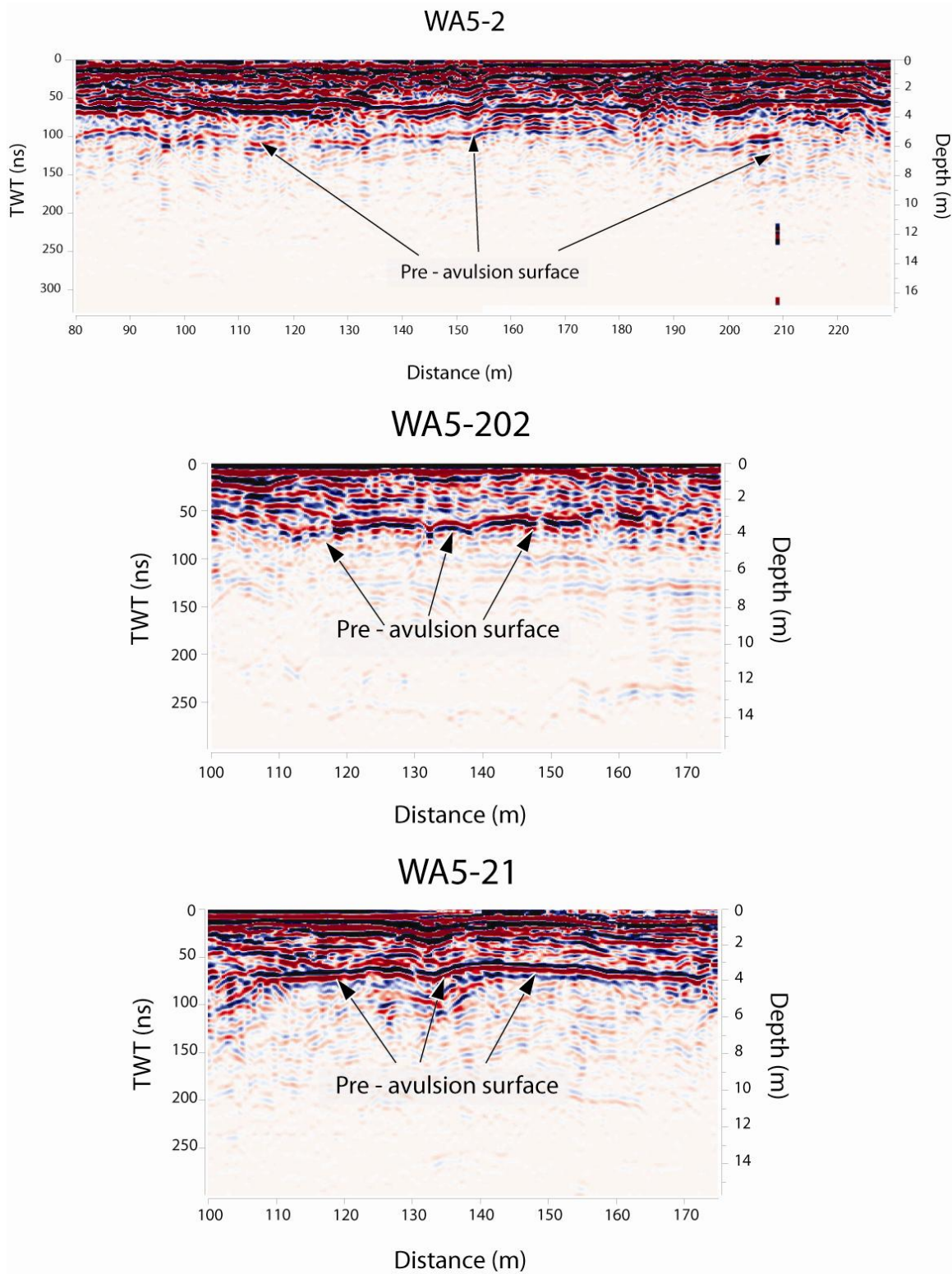


Figure 2.5. Three GPR profiles showing the strong horizontal reflector interpreted to be the pre-avulsion fan surface. See Figure 2.3 for locations of survey lines. TWT = two-way travel time.

2.4.2 Sediment Volume Calculations

The stop banks confined sedimentation following the 1967 avulsion event to a well defined area, which allows an estimate to be made of the total volume of sediment deposited since that date. In order to calculate this volume, I divided the affected area into the following five areas based on geomorphology and proximity to the stop banks: (1) the active fan north of the SH6 bridge; (2) the pre-1967 and current braidplain; (3) the Lake Wahapo delta; (4) the inactive fan outside the stop bank on the east (OSE), and (5) the inactive fan outside the stop bank on the west (OSW) (Fig. 2.3). Areas 4 and 5 were part of the active fan surface until the mid-1980s, when the present stop banks were built and sediment supply to these areas was cut-off.

2.4.2.1 Active fan north of SH6 bridge

The current active fan is bordered by stop banks on the east and west, the Okarito moraine on the north, and 1967 margin of Lake Wahapo on the northwest. The southern boundary of this sector is more poorly constrained, because the active fan is continuous with the floodplain of Waitanitaona River above the SH6 bridge. I chose as a southern boundary the point where the river avulsed in 1967. The total area of the current active fan, measured on the 1985 aerial photograph is ~3.2 km². The 1967 surface, based on GPR data, is almost everywhere 4 m below the modern surface, although along line WA5-2 (Fig. 2.3) it is up to 6 m deep. The total volume of sediment in this geomorphic zone is approximately 1.3×10^7 m³ (Table 2.1)

2.4.2.2 Braidplain south of SH6 bridge

The sector between the SH6 bridge and Gaunt Creek is a composite of the pre-

1967 and the current braidplain. It has an area of 1.9 km². No GPR data were collected in this area, so the depth to the pre-1967 surface is assumed to be the same as that on the active fan north of the SH6 bridge. The total volume for this sector is about 7.6 x 10⁶ m³ (Table 2.3)

2.4.2.3 Lake Wahapo delta

Since its inception in 1967, a delta has prograded about 567 m into Lake Wahapo, at an average rate of 13.8 m a⁻¹ (Fig. 2.6). I divided the volume of the delta sediment pile into two parts, the delta plain, which is above current lake level and the subaqueous delta

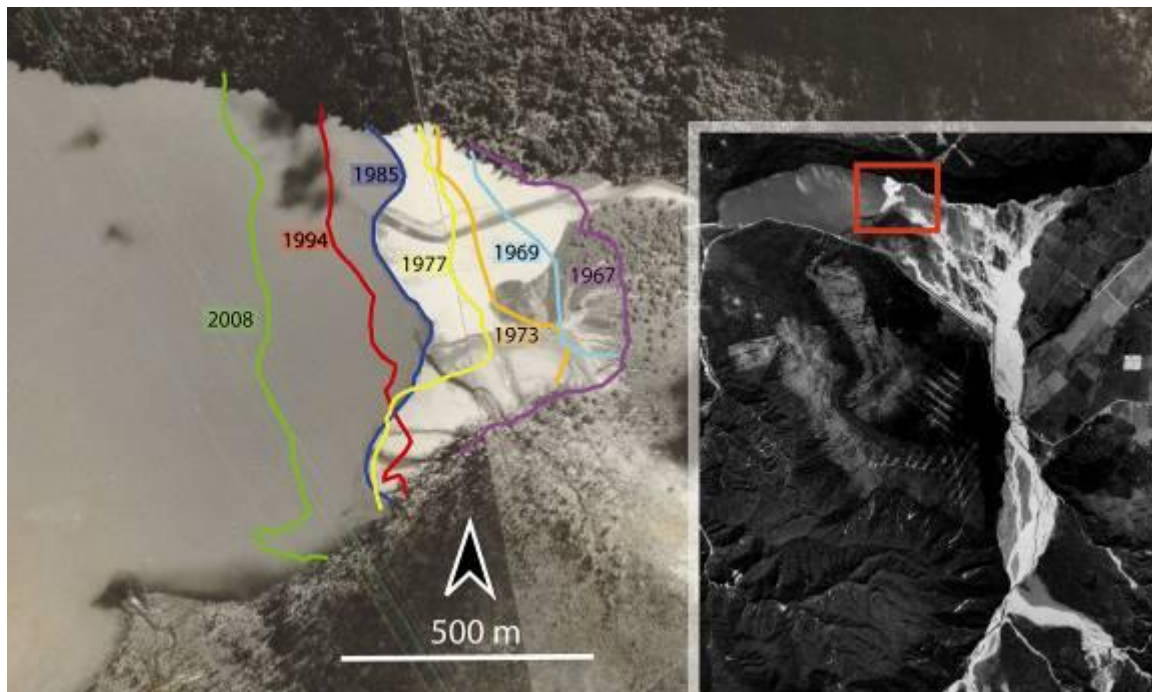


Figure 2.6. Growth of the Lake Wahapo delta since 1967 based on interpretation of aerial photographs. Base photograph, taken in 1985, provided courtesy of the West Coast Regional Council. The inset photograph was also taken in 1985 and provided courtesy of Land Information New Zealand.

wedge (represented by the clinoforms). The area of the delta plain is 0.29 km². Based on the GPR data, the estimated average thickness of the deltaic sequence is 5 m, thus the

volume of sediment is $1.4 \times 10^6 \text{ m}^3$ (Table 2.1). The volume of the subaqueous wedge had to be inferred because no data are available to constrain the thickness of the sediments deposited in the lake. The tangential reflectors of the delta terminate on an

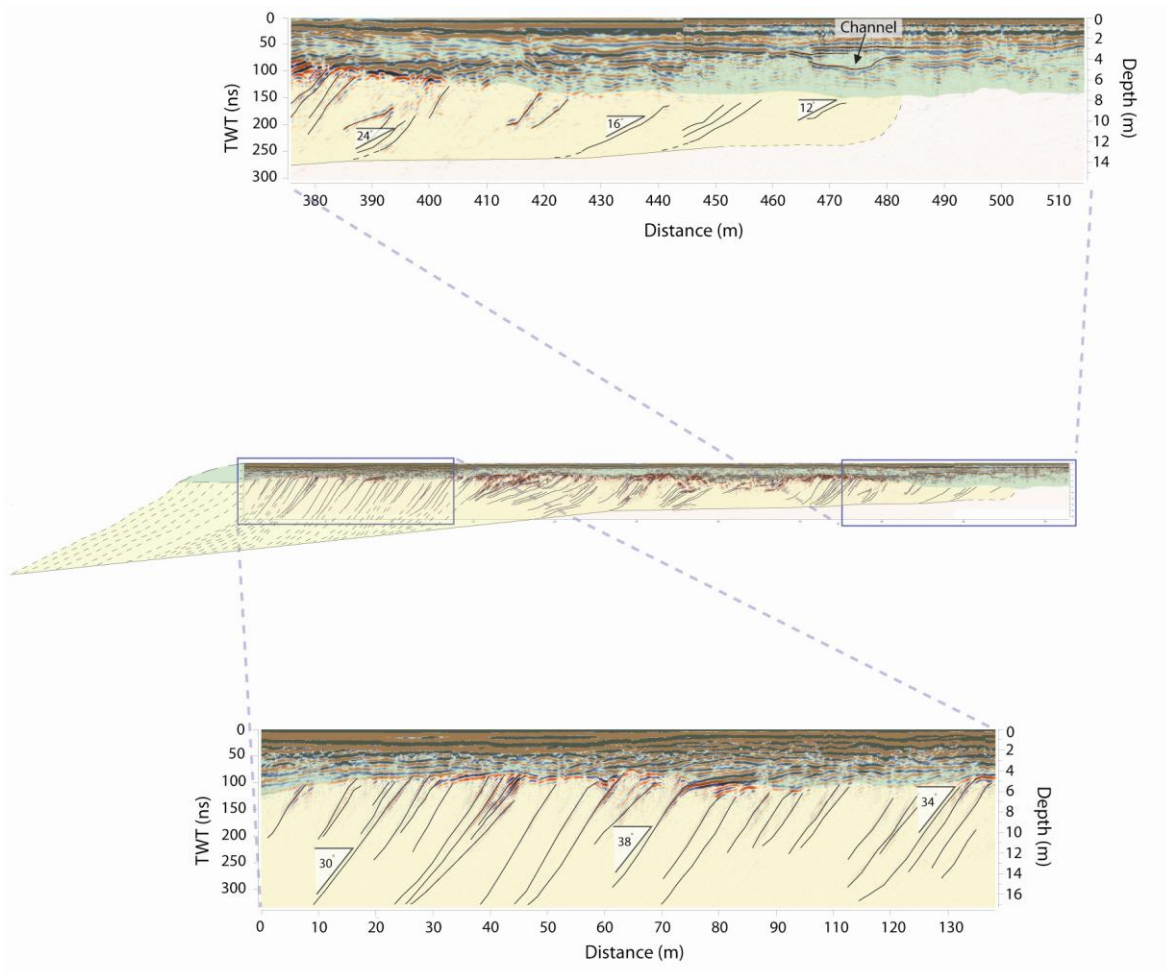


Figure 2.7. GPR profile and interpretive sketch of the Lake Wahapo delta; vertical exaggeration is 2x; specific values on the figure are the true dip angles. Clinoform dip angles range from 12 to 38°. The terminations of the clinoforms cannot be seen at the present lakeshore (left) due to the thickness of the foreset beds in that area. The clinoforms nearer the 1967 lakeshore (right) are conformable to the lake bottom at that time.

unseen horizon, which I interpret to be the original lake bottom. I projected the lake bottom out to meet the lowest dipping reflector to create a realistic wedge model (Fig.

2.7). The area of this wedge is $4.4 \times 10^5 \text{ m}^2$. Based on this wedge model, I estimate the average thickness of the deltaic sequence to be 5 m. The volume of sediment deposited in the wedge thus is about $2.2 \times 10^6 \text{ m}^3$ and the estimated total volume of sediment deposited in the delta sector is about $3.6 \times 10^6 \text{ m}^3$ (Table 2.1).

2.4.2.4 Inactive fan outside the existing stop banks on the east and west

Stop banks were constructed on the fan in the early 1980s to prevent Waitangitaona River from returning to its pre-1967 course. Between 1967 and the time the stop banks were built, areas both to the east and west received sediment. No GPR data were collected in these areas, but the depth of sediment was determined from 1) the elevation differences on opposite sides of both the east and west stop banks and 2) the thickness of post-1967 sediment inside the two stop banks, as determined from the GPR

Table 2.1. Sediment volume estimates for the five areas of the Waitangitaona fan and delta.

| Sector | Average depth (m) | Area (m ²) | Volume (m ³) | Percent of total area | Percent of total volume |
|--------------------------------|-------------------|-------------------------------------|-------------------------------------|-----------------------|-------------------------|
| Active fan north of SH6 bridge | 4.0 | 3.2×10^6 | 1.3×10^7 | 47.4 | 50.0 |
| Braidplain south of SH6 bridge | 4.0 | 1.9×10^6 | 7.6×10^6 | 28.0 | 29.5 |
| Delta plain | 5.0 | 2.9×10^5 | 1.4×10^6 | 4.5 | 5.6 |
| Subaqueous delta (wedge) | 5.0 | 4.4×10^5 | 2.2×10^6 | 6.5 | 8.6 |
| Outside stop bank (east) | 1.75 | 7.9×10^5 | 1.4×10^6 | 11.8 | 5.4 |
| Outside stop bank (west) | 1.75 | 1.3×10^5 | 2.3×10^5 | 1.9 | 0.9 |
| Total | | 6.7×10^6 | 2.6×10^7 | | |

survey. The average difference in elevation is 2.25 m, which, together with a 4-m thickness inside the stop banks, suggests a sediment depth of 1.75 m for both sections. The areas of OSE and OSW are, respectively, 7.9×10^5 and 1.3×10^5 m². The corresponding volumes are 1.39×10^5 m³ and 2.27×10^5 m³ (Table 2.1).

2.4.3 *Backhoe Trenching*

Backhoe trenching provided information on the elevation of the local water table and the rooting level of a buried in-situ tree. The backhoe operator was able to extend the edge of the bucket under the bole of the tree along survey line WA5-17 (Fig. 2.3). The base of the tree represents the 1967 pre-avulsion surface. The depth to the rooting level was 3.8 m, which is in agreement with the results from the GPR survey.

2.4.4 *Sediment Yield and Denudation Rate*

The total volume of post-1967 coarse sediment (gravel and sand) is 2.6×10^7 m³. This volume corresponds to an apparent average coarse sediment discharge of 625,000 m³ a⁻¹ and a specific coarse sediment yield for the watershed of 30,000 t km⁻² a⁻¹ (Table 2.2). Annual sediment discharge was calculated by dividing the total post-1967 sediment volume by the number of years over which it accumulated (41 years). The specific sediment yield was determined by multiplying the sediment discharge by the bulk density, in this case 1.8 t m⁻³ and then dividing this value by the catchment area: sediment yield = (sediment discharge x bulk density) / catchment area. Three-fourths of this sediment was deposited on the active fan surface between Gaunt Creek and the 1967 shoreline of Lake

Wahapo. This part of the fan has aggraded 3.69 m since 1967, an average rate of 0.09 m a⁻¹.

Changes in elevation of the bed of Waitangitaona River have been monitored by the West Coast Regional Council (WCRC) using cross-section survey data acquired in 1982, 1992, and 2001. The data show that the channel aggraded about 2 m between 1982 and 2001 (Korup 2004; Korup et al. 2004), an average rate of about 0.11 m a⁻¹. This rate is agreement with the longer term average determined in this study.

Table 2.2. Coarse sediment yield for the Waitangitaona River watershed.

| | |
|--|---------|
| Apparent coarse sediment discharge (m³ a⁻¹) | 625,000 |
| Specific coarse sediment yield (t km⁻² a⁻¹) | 30,000 |
| Denudation rate (mm a¹) | 5.1 |
| Fan aggradation (m a⁻¹) | 0.09 |

It is necessary to consider the porosity of the deposited sediments when determining denudation rates from sediment yield values. Assuming that the coarse sediment that was eroded and then deposited has a porosity of 0.39 (Kamann et al. 2007), the average annual denudation rate for the watershed based on the coarse sediment fraction alone is 5.1 mm a⁻¹. It is clear, however, that the actual denudation rate is significantly greater than this because I do not account for silt and clay in my sediment budget. It is also clear that denudation differs widely throughout the watershed, because much of the sediment delivered to the coastal plain in South Westland is derived from small areas affected by landslides.

2.5 Discussion

2.5.1 Sediment Yields and Denudation Rates

Specific sediment yields and denudation rates for the Waitangitaona River watershed have been estimated by Griffiths and McSaveney (1986), Hovius et al. (1997), and Korup et al. (2004) (Table 2.3). Griffiths and McSaveney (1986) applied a simple equation based on trapping efficiency to estimate sediment yield. Their estimate of trapping efficiency of 0.53 was based on sediment samples taken from the Lake Wahapo delta and the Gaunt Creek slip. For the period 1968-1984, they calculated a denudation rate of 4.6 mm a^{-1} and a specific sediment yield of $12,500 \text{ t km}^{-2} \text{ a}^{-1}$.

Hovius et al. (1997) estimated denudation rates in the Waitangitaona River watershed by identifying, characterizing, and mapping landslides on aerial photographs. Their estimates of denudation and sediment yield over the entire watershed for the period 1948-1985 are, respectively, 18.1 mm a^{-1} and $1.1 \times 10^6 \text{ m}^3 \text{ a}^{-1}$. They applied a similar approach to 13 nearby catchments in South Westland, all of which are larger than the Waitangitaona River watershed or have a significant glacier in their headwaters (e.g., Franz Joseph Glacier in the Waiho River catchment). Six of the 13 catchments have higher sediment yields than the Waitangitaona River watershed.

The average denudation rate calculated by Hovius et al. (1997) more than three times that of my estimate and four to nearly five times that of other estimates summarized in Table 2.3. The estimate of Hovius et al. (1997) is based on landslides mapped using aerial photographs. It thus includes fine as well as coarse sediment. This

study, however, only accounts for the coarse fraction of the denuded sediment and therefore the estimate of the average denudation rate is lower.

Table 2.3. Comparison of estimates of sediment yield and denudation rates for the Waitangitaona River watershed.

| Study | Sediment discharge (m³ a⁻¹) | Average sediment yield (t km⁻² a⁻¹) | Denudation rate (mm a⁻¹) | Period |
|--------------------------------|--|--|--|---------------|
| Griffiths and McSaveney (1986) | 347,000 | 12,500 | 4.6 | 1968-1984 |
| Hovius et al. (1997) | 1,100,000 | 45,400 | 18.1 | 1948-1986 |
| Korup et al. (2004) | 289,000 | 11,500 | 3.9 | 1968-2001 |
| This study | 625,000 | 30,000 | 5.1 | 1967-2008 |

Korup et al. (2004) used digital elevation models and estimated trapping efficiency value to calculate the amount of sediment produced by landslides in the Waitangitaona catchment. Their estimates of sediment discharge and average sediment yield were derived from the same deposit surface that I used. Their estimates of sediment yield and denudation are similar to those of Griffiths and McSaveney (1986), but span a longer period.

Hicks et al. (1996) estimated specific sediment yields for South Westland watersheds, although not the Waitangitaona River catchment. Their values range over four orders of magnitude, from 1.7 to 29,600 t km⁻² a⁻¹. The specific sediment yield that I calculated for the Waitangitaona catchment is at the upper end of this range and likely reflects episodic sediment delivery from the Gaunt Creek slip rather than uniform denudation of the entire catchment.

Korup et al. (2004) attempted to isolate landslide-derived sediment from sediment produced by steady-state erosion in the Waitangitaona River watershed. A comparison of the two sources of sediment is difficult, because the short historical observation period confounds attempts to reliably quantify average recurrence intervals for landslides of different sizes (Korup et al. 2004).

Davies and Korup (2007) estimated a sediment discharge rate from the Gaunt Creek slip of $1.6 \times 10^5 \text{ m}^3 \text{ a}^{-1}$. Extrapolated over 40 years, this rate would provide about 39 percent of the total sediment volume determined in this study.

2.5.2 Error and Uncertainty

I acknowledge several assumptions and sources of error. I assumed that the depth of post-1967 sediment in each of the five parts of the Waitangitona fan is constant. A range of depths, however, is likely within each of the five areas. The mean sediment depth for each area is based on the GPR data and may be in error by up to 15 percent.

The study does not take into account the suspended load transported by Waitangitaona River and deposited in Lake Wahapo. The ratio of suspended sediment load to the total sediment load can range from 10 to 90 percent for mountain streams (Lauffer and Sommer 1982; Whittaker 1987; Diez et al. 1988; Billi et al. 1998).

2.5.3 Threat to the SH6 Bridge and Wahapo Dam

Continued aggradation of the Waitangitaona River floodplain poses a threat to the SH6 bridge. As sediment continues to accumulate on the floodplain and as the river channel lengths due to extension of the Lake Wahapo delta, the freeboard between the river channel and the bridge deck will decrease. With a reduced freeboard, the bridge is

more vulnerable to damage or destruction during an extreme flood. The bridge is vulnerable, however, even without continued aggradation. A shift in the location of the channel beneath the bridge, accompanied by incision, could erode the bridge piers. In 1982, channel incision undermined a pier of the SH6 bridge over Waitangitaona River, causing the collapse of the pier and two spans (Griffiths and McSaveney 1986).

A small hydroelectric facility was built at the outlet of Lake Wahapo after the Waitangitaona River avulsion to take advantage of the greatly increased inflow into lake. In an effort to understand the how fast the lake is filling with sediment, I estimated the volume of the lake by multiplying the area of the lake by my estimate of average water depth based on the wedge model described in section 2.4.2.3. Since 1967, about $7.9 \times 10^6 \text{ m}^3$ of sediment have been deposited in the lake and the delta has advanced nearly 600 m towards the lake outlet. At present rates, Lake Wahapo will become completely filled with sediment in about 300 years, but the hydroelectric facility will become inoperable long before this due to reduced storage capacity and the passage of silt and clay through the turbines. My estimate of filling time is too high because I was unable to include the unknown amount of suspended sediment that has accumulated on the lake bottom. A future study of the volume of sediment that has been deposited on the lake bottom since 1967 would provide a better estimate of the time that will pass before the lake becomes filled with sediment.

2.6 Conclusions

The unique combination of an avulsion event and subsequent flow confinement by stop banks facilitated quantification of sediment deposited on the Waitangitaona River fan since 1967. I performed a ground-penetrating radar survey of the fan to determine the

thickness of sediment deposited above the 1967, pre-avulsion surface. The survey, in conjunction with comparative air photo analysis, a differential GPS survey, and backhoe trenching, allowed me to estimate the total volume of sediment deposited on the confined floodplain since 1967. I estimated average annual sediment discharge and denudation based on this sediment volume. The values are within the range of those reported in three previous studies of the Waitangitaona River catchment. The Gaunt Creek slip is the single most important source of sediment to Waitangitaona River fan. It is responsible for over one-third of the sediment delivered to the fan and caused the avulsion that made this study possible.

CHAPTER 3: DOWNSTREAM EFFECTS OF THE 1999 POERUA RIVER LANDSLIDE DAM FAILURE

3.1 Introduction

Large pulses of sediment introduced to a fluvial system can have a dramatic impact on the morphology of the stream. Landslides are an important source of sediment to streams and, in some instances, block drainage, creating upvalley lakes. Unlike engineered dams, landslide barriers consist of unconsolidated and unsorted materials that are susceptible to failure by piping, collapse, and incision by overflowing waters (Costa 1985; Costa and Schuster 1988). Failures of landslide dams may cause catastrophic flooding, aggradation, and avulsion, and they commonly increase the possibility of subsequent flooding in downstream areas (Hancox et al. 2005).

Many rivers in South Westland, New Zealand, are subject to blockage by landslides, followed by failure and large fluxes of sediment below the dams. One such river, Poerua River, is the subject of this chapter. On 6 October 1999, a large ($10\text{-}15 \times 10^6 \text{ m}^3$) rock slope failure on the flank of Mt. Adams (2130 m asl) in the Southern Alps blocked Poerua River 11 km upstream of the State Highway 6 (SH6) bridge (Fig. 3.1). The fragmented rock mass descended almost 1800 m into the Poerua River gorge, creating a 80-100-m-high dam that completely stemmed the flow of the river (Hancox et al. 2005). Between 5 and $7 \times 10^6 \text{ m}^3$ of water accumulated behind the barrier prior to overtopping late on 7 October 1999. The dam breached during a rainstorm early on the morning of 12 October 1999. The resulting outburst flood had a peak discharge of 2000-3000 $\text{m}^3 \text{ s}^{-1}$ at the breach and a discharge of 800-1000 $\text{m}^3 \text{ s}^{-1}$ at the SH6 bridge (Hancox

et al. 2005). Incision of the dam was accompanied by the transfer of large amounts of sand and gravel downstream, both in the mountain valley and on the fan on the coastal plain farther west. Korup et al. (2004) calculated that $7.37 \times 10^6 \text{ m}^3$ of sediment had been deposited in the lower gorge and on the fan by February 2002. Aggradation of the stream channel due to deposition of sediment from the outburst flood on the Poerua River fan led to an avulsion in April 2001 that destroyed 0.9 km^2 of farmland (Korup 2004). By April 2003, the river had returned to its pre-avulsion channel in the center of the valley (Hancox et al. 2005).

The immediate and subsequent (up to 2005) effects of the dam break flood have been previously reported (Hancox et al. 1999, 2005; Korup et al. 2004; Davies and Korup 2007) and will not be repeated here. Instead, I focus on changes to Poerua River between 2005 and 2008, including changes in floodplain level at 34 surveyed cross-sections and changes in sediment input and conveyance along the lower part of the river west of the mountain front. The objective of this study is to document changes in sediment flux towards the end of the first decade following the landslide.

3.2 Study Area

The study area is an 11-km reach of Poerua River extending from the mouth of the Poerua River gorge at the mountain front to a point 5 km downstream of the SH6 bridge (Fig. 3.1). The Poerua River gorge has steep (average = $38\text{--}40^\circ$) slopes and relief up to 1800 m. The valley walls are densely vegetated, except at sites where landslides and debris flows have occurred in the past several decades. Poerua River is confined in this steep-sided valley as far west as the mountain front; beyond the mountain front, the

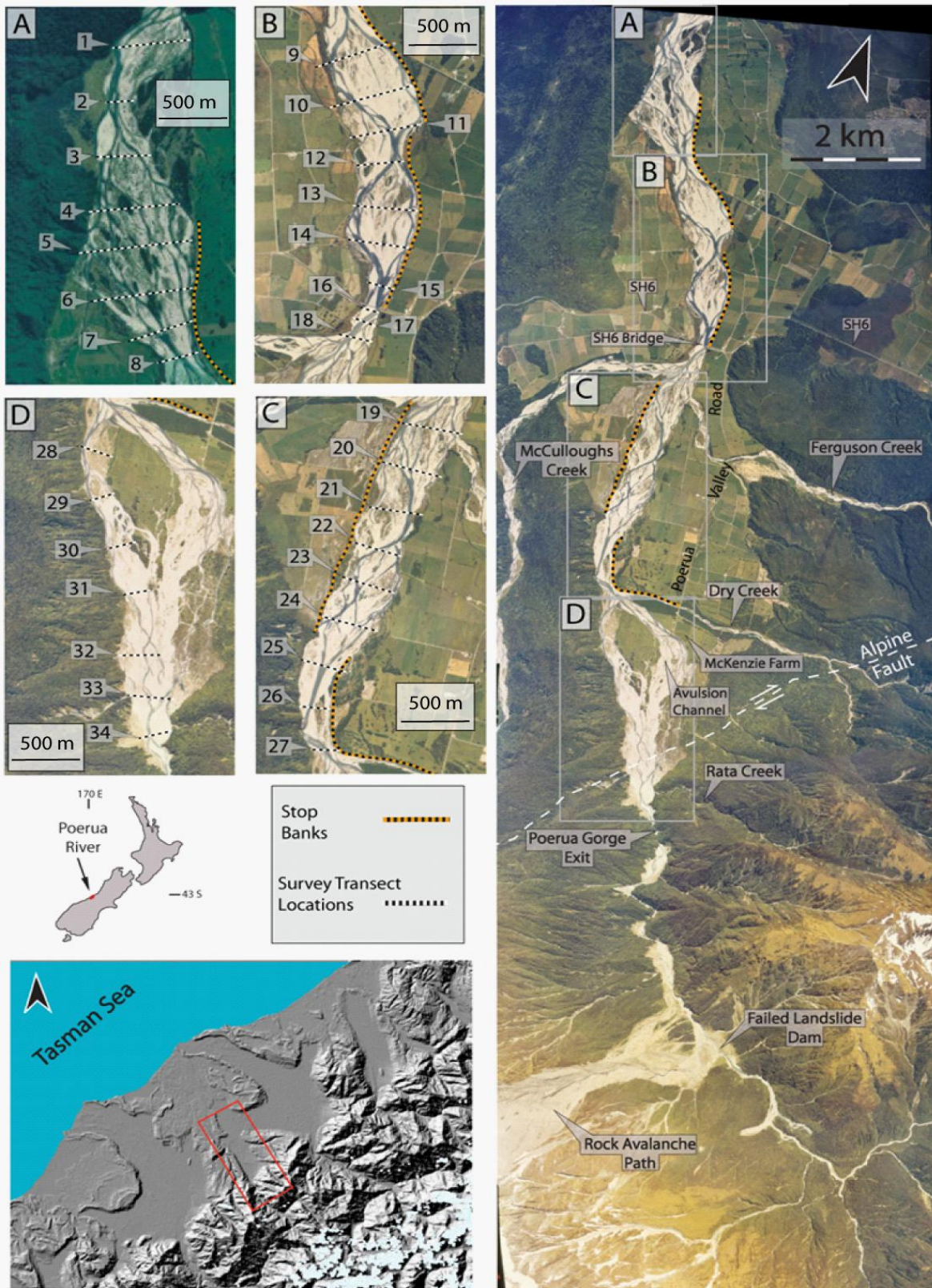


Figure 3.1. Right: Aerial photograph of the study area, showing the site of the 6 October 1999 Mt. Adams rock avalanche and downstream surveyed reaches of the Poerua River in South Westland, New Zealand. Inset shaded relief map shows the location of the aerial photograph (red rectangle). A, B, C, D) 34 surveyed cross-sections. (Aerial photograph (flown by Air Logistics Ltd) provided by Tim Davies.)

river flows across a low-gradient fan onto the coastal lowland bordering Tasman Sea. This part of the river is characterized by a braided planform with gravel-armoured channels. In this area, the floodplain is bordered by large lateral moraines deposited during several Pleistocene glaciations. Precipitation in the headwaters of the watershed is up to 11 m a^{-1} (Griffiths and McSaveney 1986), with snow dominating at higher elevations during the austral winter (Hancox et al. 2005). Bedrock consists of metamorphosed and deformed greywacke; the metamorphic grade increases close to the Alpine Fault (Norris and Cooper 1997).

3.3 Methods

Changes in the level of the Poerua River channel were monitored at 34 cross-sections across the channel by the West Coast Regional Council (WCRC) between 1999



Figure 3.2. Topographic survey of Poerua River just above the SH6 bridge. Data are being gathered by the rover component of the Top Con dGPS system. Mt. Adams is the high peak in the background. (Photograph by Michelle Hanson, 2008).

and 2005. These data have been used to estimate the amount of sediment delivered and transported along the surveyed reach of the channel and the amount of aggradation or degradation at each cross-section. Data were first collected in May 1999 before the dam-break flood, and the cross-sections were resurveyed in June 2000, December 2000, January 2001, February 2002, August 2003, and July 2005. All 34 cross-sections were surveyed only in February 2002, July 2005, and May 2008. The other surveys included 17-33 cross-sections, all located between the SH6 bridge and the mountain front.

I re-surveyed the 34 cross-sections in May 2008 using a TopCon dGPS HiPer L1 system and a local datum (NZGS 1949) (Fig. 3.2). The dGPS system consists of a base station and a rover. An iron pipe that the original surveyors used to mark the beginning of each cross-section was used to geo-reference the section. In order to check the accuracy of the dGPS system before each survey, neighbouring iron pipes were surveyed. I collected survey points in real time kinematic (RTK) mode, which provides an accuracy of 10 mm in the horizontal plane and 15 mm vertically. Bed elevation data were automatically collected, arbitrarily, every 2 m along the profiles; additional data were collected manually at points of significant change in elevation between the 2-m collection points. Methods employed to ensure the most accurate results included using a large number of satellites for locations, optimal dilution of precision (DOP), and lengthy occupation of measurement positions.

I derived the average elevation along each cross-section from the mean of all data points collected over each transect. Volumetric estimates of the net sediment flux were obtained by multiplying the mean elevation by the area between each cross-section.

3.4 Results

Because all 34 cross-sections were surveyed only in February 2002, July 2005, and May 2008, I focus here on an analysis of floodplain and active channel levels and sediment volume changes at cross-sections 17-34, which were surveyed eight times between 1999 and 2008. The initial survey in May of 1999, approximately five months before the rock avalanche, provided the pre-event floodplain elevations to which all the other surveys were compared.

3.4.1 Floodplain Elevation Changes

3.4.1.1 Changes in cross-sections 17-34 since 1999

Floodplain elevations generally remained above their May 1999 levels between the time of the landslide and at least 2005 (Table 3.1; Fig. 3.3). Hancox et al. (2005) reported that from 1999 to 2002, the cross-section surveys show a substantial change in the floodplain, with a maximum elevation rise of 4 m. Aggradation of the fan was continuing in 2005, causing damage to farmland (Davies and Korup 2007). Between 2005 and 2008, however, floodplain elevations dropped below their pre-landslide levels at six cross-sections, with the greatest decrease (0.38 m +/- 15 mm) recorded at cross-section 21. The results indicate that the distal part of the fan was incised between 2005 and 2008.

Further evidence for recent fan incision is provided by comparing bed elevations between successive surveys. The data show an increasing number of negative changes (lowering of floodplain levels) over time (Table 3.2; Fig. 3.4). Not all cross-sections,

Table 3.1. Change in mean floodplain elevations relative to May 1999 (in m \pm 15 mm).

| Cross- Section # | Distance above SH6 bridge (km) | Survey date | | | | | |
|---------------------|-----------------------------------|-------------|--------|--------|--------|--------|--------|
| | | Jun-00 | Dec-00 | Feb-02 | Aug-03 | Jul-05 | May-08 |
| 17 | 0.06 | 0.12 | 0.10 | 0.29 | 0.01 | 1.00 | -0.05 |
| 18 | 0.24 | 0.08 | 0.10 | 1.17 | 1.18 | 0.94 | 0.21 |
| 19 | 0.63 | 0.28 | 0.15 | 0.71 | 0.30 | 0.32 | 0.24 |
| 20 | 1.05 | -0.03 | 0.20 | 0.29 | 0.00 | 0.04 | -0.14 |
| 21 | 1.37 | 0.25 | 0.32 | 0.40 | 0.29 | 0.34 | -0.38 |
| 22 | 1.78 | 0.15 | 0.24 | 0.25 | 0.38 | 0.31 | -0.25 |
| 23 | 2.17 | 0.19 | 0.23 | 0.38 | 0.30 | 0.40 | 0.36 |
| 24 | 2.58 | 0.35 | 0.19 | 0.17 | 0.41 | 0.32 | 0.48 |
| 25 | 2.97 | 0.44 | 0.50 | 0.65 | 0.69 | 0.65 | 0.46 |
| 26 | 3.37 | 0.32 | 0.37 | 0.32 | 0.16 | 0.10 | -0.07 |
| 27 | 3.77 | 0.33 | 0.32 | 0.33 | 0.35 | 0.57 | -0.07 |
| 28 | 4.17 | 0.33 | 0.20 | 0.41 | 0.34 | 0.23 | 0.31 |
| 29 | 4.57 | 1.02 | 1.01 | 1.18 | 0.99 | 0.85 | 0.74 |
| 30 | 4.99 | 0.47 | 0.70 | 0.88 | 0.81 | 0.89 | 0.98 |
| 31 | 5.46 | 1.09 | 1.29 | 1.68 | 1.91 | 2.04 | 2.02 |
| 32 | 5.98 | 3.18 | 3.34 | 3.25 | 4.11 | 4.76 | 4.00 |
| 33 | 6.34 | 1.70 | 2.13 | 2.13 | 2.46 | 3.31 | 3.92 |
| 34 | 6.79 | 2.27 | 2.58 | 2.93 | 4.72 | 2.93 | 0.79 |

however, lowered in level. Some cross-sections showed an increase in mean bed level. Cross-section 33 had the largest increase – 0.61 m. In addition, the bed level at cross-section 33 increased between each survey after June 2000, except for the period between December 2000 and February 2002 when it was unchanged (Table 3.2). The consistent increase in mean bed level for section 33 could be due to rapid trenching of the fan between the initial deposition of outburst flood sediment and the first survey in June 2000. In addition, only the active channel, not the inactive post-dam break terraces, was surveyed, and only the current active channel has been aggrading.

The floodplain at cross-section 34, nearest the mountain front, was highest in 2003 and has lowered since then. It experienced the largest decrease (2.15 m) of all

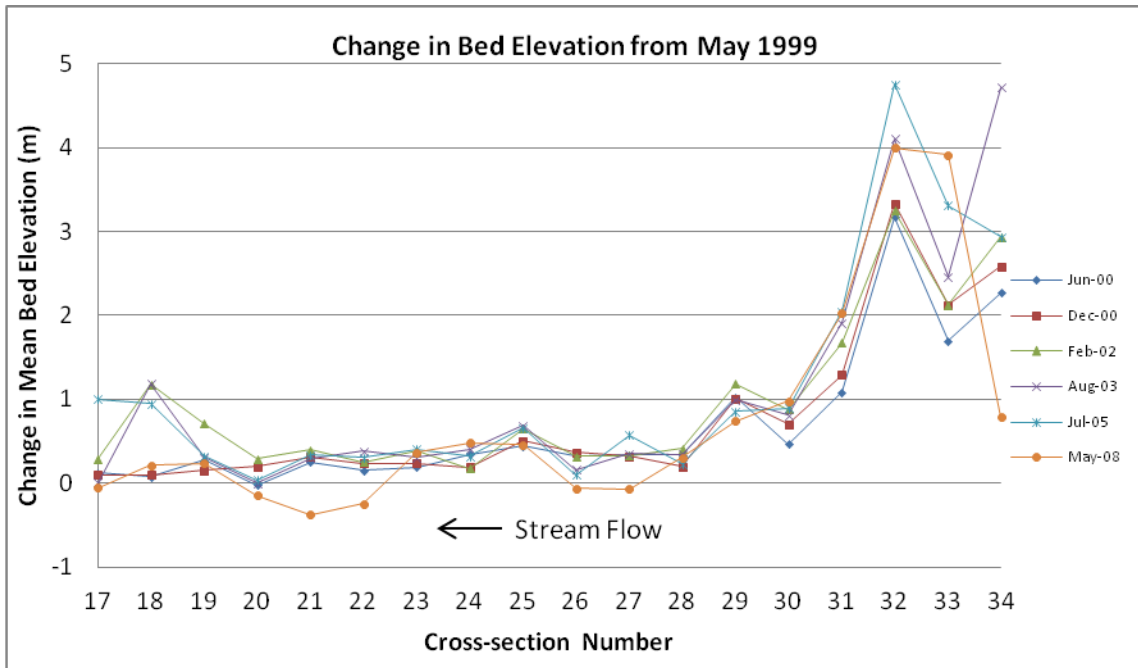


Figure 3.3. Changes in elevation of the floodplain of Poerua River between the mountain front and the SH6 bridge, relative to May 1999. See Figure 3.1 for location of sections and Table 3.1 for distances of sections of SH6 bridge.

cross-sections between 2005 and 2008. Davies and Korup (2007) reported that the sediment pulse resulting from the breach of the Poerua River landslide dam passed the mountain front at cross-section 34 in 2005. They attributed the change from aggradation to incision at this site to trenching of the fan head, which had been oversteepened due to aggradation from the initial dam break flood wave (Fig. 3.5). The aggraded fan head steepened, and after five years the river incised the fan. However, at cross-section 33, 454 m downstream from cross-section 34, the bed was higher than at any time since the landslide. The difference in response at the two cross-sections can be explained by a downstream migration of the sediment pulse from cross-section 34 to cross-section 33 between 2005 and 2008.

Table 3.2. Change in mean bed elevation from the preceding survey (in m \pm 15 mm).

| Cross-section # | Distance above SH6 bridge (km) | Survey date | | | | | |
|-----------------|--------------------------------|-------------|--------|--------|--------|--------|--------|
| | | Jun-00 | Dec-00 | Feb-02 | Aug-03 | Jul-05 | May-08 |
| 17 | 0.06 | 0.12 | -0.02 | 0.19 | -0.28 | 0.99 | -1.05 |
| 18 | 0.24 | 0.08 | 0.02 | 1.08 | 0.01 | -0.24 | -0.73 |
| 19 | 0.63 | 0.28 | -0.12 | 0.56 | -0.41 | 0.02 | -0.08 |
| 20 | 1.05 | -0.03 | 0.23 | 0.09 | -0.29 | 0.04 | -0.18 |
| 21 | 1.37 | 0.25 | 0.06 | 0.08 | -0.10 | 0.04 | -0.72 |
| 22 | 1.78 | 0.15 | 0.09 | 0.01 | 0.13 | -0.07 | -0.56 |
| 23 | 2.17 | 0.19 | 0.04 | 0.15 | -0.08 | 0.10 | -0.04 |
| 24 | 2.58 | 0.35 | -0.16 | -0.01 | 0.23 | -0.09 | 0.16 |
| 25 | 2.97 | 0.44 | 0.06 | 0.15 | 0.04 | -0.04 | -0.20 |
| 26 | 3.37 | 0.32 | 0.05 | -0.05 | -0.16 | -0.06 | -0.16 |
| 27 | 3.77 | 0.33 | -0.01 | 0.01 | 0.03 | 0.22 | -0.64 |
| 28 | 4.17 | 0.33 | -0.14 | 0.22 | -0.07 | -0.11 | 0.08 |
| 29 | 4.57 | 1.02 | -0.01 | 0.17 | -0.19 | -0.14 | -0.11 |
| 30 | 4.99 | 0.47 | 0.24 | 0.17 | -0.06 | 0.08 | 0.08 |
| 31 | 5.46 | 1.09 | 0.20 | 0.39 | 0.24 | 0.13 | -0.02 |
| 32 | 5.98 | 3.18 | 0.16 | -0.09 | 0.86 | 0.65 | -0.76 |
| 33 | 6.34 | 1.70 | 0.43 | 0.00 | 0.33 | 0.85 | 0.61 |
| 34 | 6.79 | 2.27 | 0.31 | 0.35 | 1.79 | -1.78 | -2.15 |

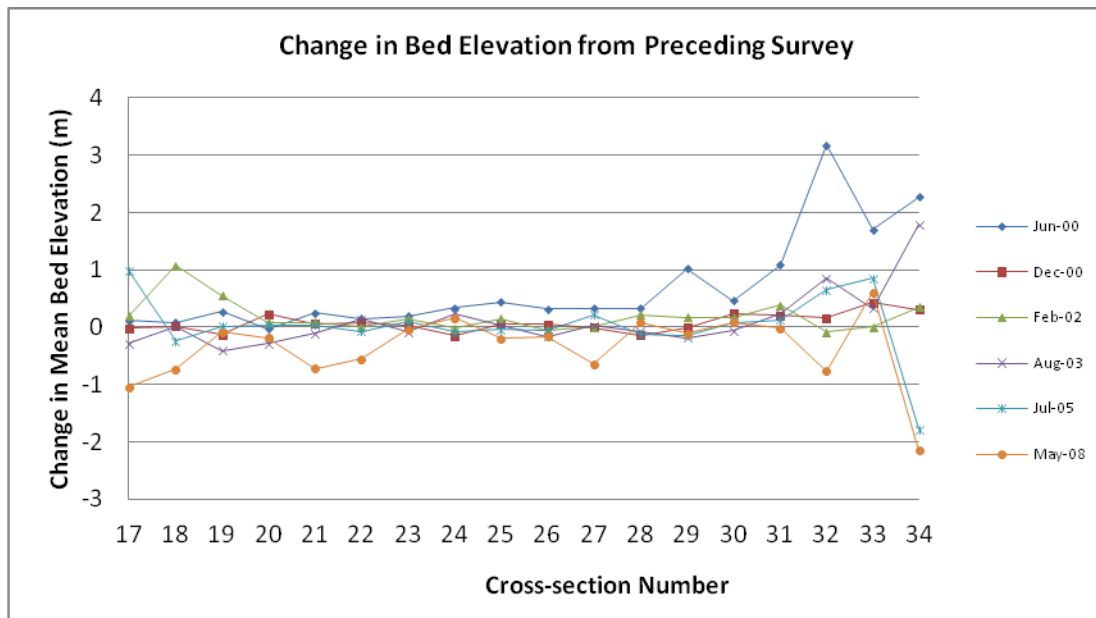


Figure 3.4. Sequence of changes in bed level at cross-sections 17 to 34 from 1999 to 2008. See Figure 3.1 for location of sections and Table 3.1 for distances of sections of SH6 bridge.

Griffiths (1993) and Madej and Ozaki (1996) forecast that aggradation at the SH6 bridge (cross-section 17) would increase in the future. However, although there was a 1 m increase in mean floodplain level at the bridge between 1999 and 2005, the 2008 survey showed a decrease of 1 m between 2005 and 2008. The decrease in floodplain level at most other sections suggests that aggradation is no longer an issue, barring another landslide or an extreme flood in the watershed.



Figure 3.5. Poerua River fan head, showing trenching of the fan near cross-section 34. The bank at the left is approximately 1.6 m high and separates the 1999 fan surface from the river level in 2008. (Photograph by Tim Davies, 2008).

In summary, most of surveyed reach of Poerua River aggraded from 1999 until 2005, but the river began to incise its floodplain in the following three years. On a smaller scale, floodplain elevation monitoring between 1999 and 2008 has shown a more

complex pattern of localized aggradation and degradation due to wave-like movements of sediment downstream (Benda and Dunne 1997).

3.4.1.2 Changes in cross-sections 1-34 since 2002

The Poerua River bed at most of the upper cross-sections (15-34) has lowered since 2002, while the river bed at the lower cross-sections (1-14) has risen (Table 3.3). The data suggest that trenching at the fan head began within several years of the landslide, but that the lower reaches are still aggrading in response to sediment being

Table 3.3. Change in mean bed elevation at all 34 cross-sections since 2002 (in m \pm 15 mm).

| From 2002 (m) | | | From the preceding survey (m) | | |
|---------------|--------|--------|-------------------------------|--------|--------|
| Cross-section | Jul-05 | May-08 | Cross-section | Jul-05 | May-08 |
| 1 | 0.02 | 0.09 | 1 | 0.02 | 0.07 |
| 2 | -0.01 | -0.25 | 2 | -0.01 | -0.24 |
| 3 | 0.02 | -0.31 | 3 | 0.02 | -0.33 |
| 4 | 0.05 | 0.06 | 4 | 0.05 | 0.01 |
| 5 | -0.13 | -0.11 | 5 | -0.13 | 0.02 |
| 6 | 0.06 | 0.15 | 6 | 0.06 | 0.09 |
| 7 | 0.18 | 0.20 | 7 | 0.18 | 0.03 |
| 8 | 0.03 | -0.11 | 8 | 0.03 | -0.15 |
| 9 | 0.05 | 0.49 | 9 | 0.05 | 0.44 |
| 10 | -0.04 | 0.12 | 10 | -0.04 | 0.15 |
| 11 | 0.03 | -0.10 | 11 | 0.03 | -0.13 |
| 12 | -0.09 | -0.11 | 12 | -0.09 | -0.02 |
| 13 | -0.13 | -0.11 | 13 | -0.13 | 0.02 |
| 14 | -0.03 | 0.07 | 14 | -0.03 | 0.10 |
| 15 | 0.05 | -0.04 | 15 | 0.05 | -0.09 |
| 16 | -0.10 | -0.64 | 16 | -0.10 | -0.54 |
| 17 | 0.71 | -0.34 | 17 | 0.71 | -1.05 |
| 18 | -0.23 | -0.96 | 18 | -0.23 | -0.73 |
| 19 | -0.39 | -0.47 | 19 | -0.39 | -0.08 |
| 20 | -0.25 | -0.43 | 20 | -0.25 | -0.18 |
| 21 | -0.06 | -0.77 | 21 | -0.06 | -0.72 |
| 22 | 0.06 | -0.50 | 22 | 0.06 | -0.56 |
| 23 | 0.02 | -0.02 | 23 | 0.02 | -0.04 |

| | | | | | |
|----|-------|-------|----|-------|-------|
| 24 | 0.15 | 0.31 | 24 | 0.15 | 0.16 |
| 25 | 0.01 | -0.19 | 25 | 0.01 | -0.20 |
| 26 | -0.22 | -0.39 | 26 | -0.22 | -0.16 |
| 27 | 0.25 | -0.40 | 27 | 0.25 | -0.64 |
| 28 | -0.19 | -0.11 | 28 | -0.19 | 0.08 |
| 29 | -0.33 | -0.44 | 29 | -0.33 | -0.11 |
| 30 | 0.02 | 0.10 | 30 | 0.02 | 0.08 |
| 31 | 0.37 | 0.35 | 31 | 0.37 | -0.02 |
| 32 | 1.51 | 0.75 | 32 | 1.51 | -0.76 |
| 33 | 1.18 | 1.79 | 33 | 1.18 | 0.61 |
| 34 | 0.00 | -2.15 | 34 | 0.00 | -2.15 |

Table 3.4. Volume (x 1000 m³) changes between adjacent cross-sections since the dam break event in May of 1999

| Between section numbers | Distance from SH6 bridge (km) | Survey date | | | | | |
|-------------------------|-------------------------------|-------------|--------|--------|--------|--------|--------|
| | | Jun-00 | Dec-00 | Feb-02 | Aug-03 | Jul-05 | May-08 |
| 17-18 | 0.24 | 3 | 3 | 6 | 8 | 23 | -2 |
| 18-19 | 0.63 | 26 | 19 | 32 | 38 | 32 | 34 |
| 19-20 | 1.05 | 23 | 33 | -10 | 6 | 19 | 8 |
| 20-21 | 1.37 | 21 | 49 | -24 | -6 | 12 | -54 |
| 21-22 | 1.78 | 35 | 48 | -3 | 18 | 23 | -63 |
| 22-23 | 2.17 | 26 | 36 | 5 | 18 | 26 | -9 |
| 23-24 | 2.58 | 49 | 39 | 42 | 65 | 62 | 72 |
| 24-25 | 2.97 | 49 | 43 | 69 | 86 | 65 | 59 |
| 25-26 | 3.37 | 44 | 50 | 64 | 56 | 39 | 23 |
| 26-27 | 3.77 | 39 | 41 | 40 | 34 | 39 | -8 |
| 27-28 | 4.17 | 30 | 23 | 24 | 21 | 26 | 11 |
| 28-29 | 4.57 | 61 | 55 | 63 | 49 | 37 | 47 |
| 29-30 | 4.99 | 71 | 82 | 88 | 74 | 72 | 85 |
| 30-31 | 5.46 | 80 | 103 | 120 | 128 | 146 | 157 |
| 31-32 | 5.98 | 293 | 318 | 409 | 434 | 465 | 414 |
| 32-33 | 6.34 | 266 | 298 | 354 | 366 | 418 | 422 |
| 33-34 | 6.79 | 206 | 245 | 272 | 207 | 213 | 235 |

transported downstream from the fan. The changes in the downstream reach, however, are not large, amounting to only decimetres since 2002. Korup et al. (2004) suggested that once the pre-landslide rate of sediment supply was re-established, the river would

incise its aggraded fan head. Given enough time and lateral erosion, the elevated fan head will be removed or greatly reduced in size.

3.4.2 Sediment Volume Changes

To calculate changes in sediment volume, I averaged the change in elevation at two adjacent cross-sections and then multiplied the average elevation change by the area between the two cross-sections. I assumed that there was a uniform sediment gain or loss over the area between the two adjacent sections.

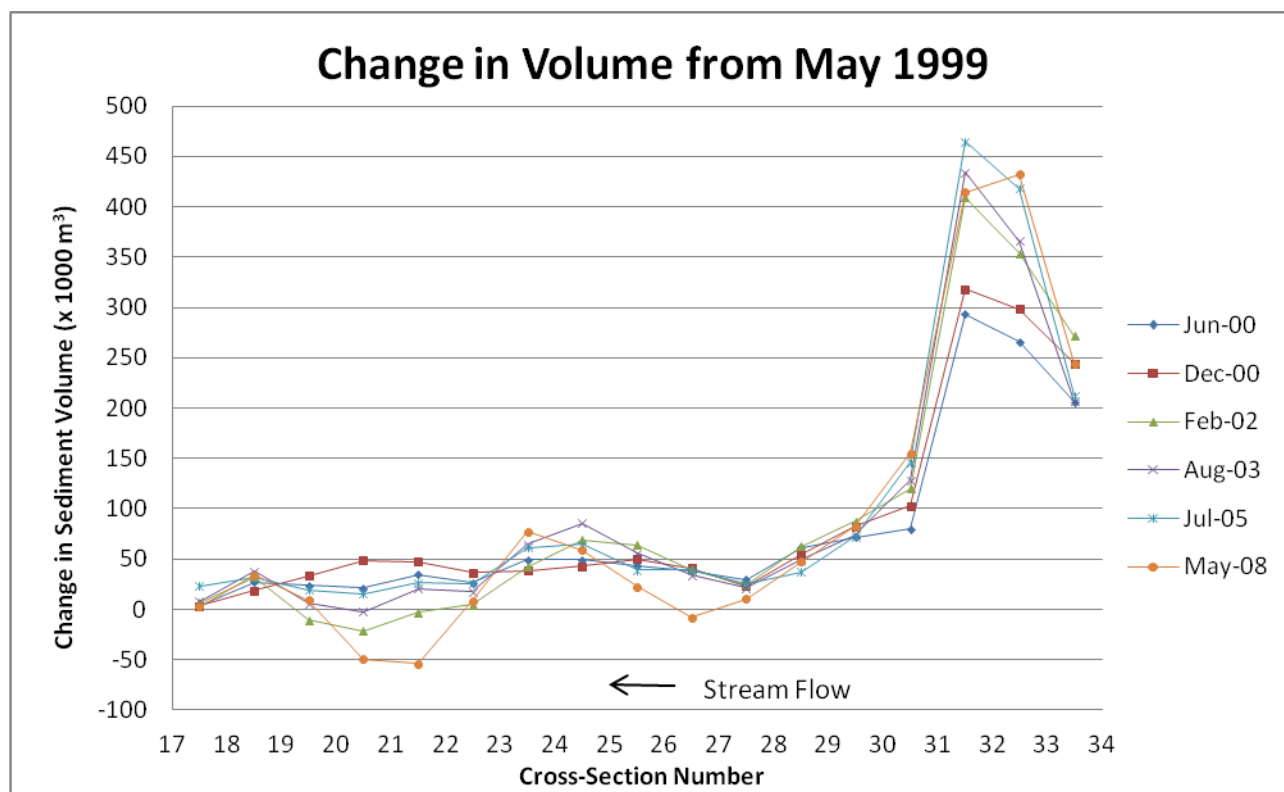


Figure 3.6. Sediment volume changes at cross-sections 17-34 between 1999 and 2008. See Figure 3.1 for location of sections and Table 3.1 for distances of sections of SH6 bridge.

There was little total loss or gain of sediment between cross-sections 17 and 34 between 1999 and 2008 (Table 3.4; Fig. 3.6). The May 2008 survey showed the largest losses, but the total amount of sediment lost from the system is tiny compared to the total amount of sediment introduced by the dam breach event. Ten of the 17 zones between cross-sections 17 and 34 show a decrease in sediment volume between 2005 and 2008. The decrease in volume was largest ($8.6 \times 10^4 \text{ m}^3$) in the zone between cross-sections 21 and 22 (1.78 km upstream of SH6 bridge) (Table 3.5; Fig. 3.7).

Seven of 17 zones between cross-sections 17 and 34 record a decrease in sediment volume between 2005 and 2008. The largest increase is just below the Poerua River gorge, 7 km upstream from the SH6 bridge. Korup et al. (2004) estimated that

Table 3.5. Volume ($\times 1000 \text{ m}^3$) changes from the preceding survey.

| Section number | Distance from SH6 Bridge (km) | Survey date | | | | | |
|----------------|-------------------------------|-------------|--------|--------|--------|--------|--------|
| | | Jun-00 | Dec-00 | Feb-02 | Aug-03 | Jul-05 | May-08 |
| 17-18 | 0.24 | 3 | -1 | 3 | 2 | 15 | -25 |
| 18-19 | 0.63 | 27 | -8 | 13 | 6 | -6 | 2 |
| 19-20 | 1.05 | 24 | 10 | -44 | 16 | 14 | -12 |
| 20-21 | 1.37 | 21 | 27 | -73 | 18 | 18 | -66 |
| 21-22 | 1.78 | 35 | 13 | -54 | 23 | 6 | -86 |
| 22-23 | 2.17 | 26 | 10 | -32 | 13 | 8 | -26 |
| 23-24 | 2.58 | 49 | -11 | 4 | 23 | -3 | 10 |
| 24-25 | 2.97 | 49 | -6 | 26 | 17 | -21 | -6 |
| 25-26 | 3.37 | 44 | 6 | 14 | -8 | -17 | -17 |
| 26-27 | 3.77 | 39 | 2 | -1 | -6 | 6 | -47 |
| 27-28 | 4.17 | 30 | -7 | 1 | -3 | 5 | -15 |
| 28-29 | 4.57 | 61 | -7 | 8 | -14 | -12 | 10 |
| 29-30 | 4.99 | 71 | 11 | 5 | -14 | -2 | 13 |
| 30-31 | 5.46 | 80 | 23 | 17 | 8 | 18 | 11 |
| 31-32 | 5.98 | 293 | 25 | 91 | 25 | 31 | -51 |
| 32-33 | 6.34 | 266 | 32 | 56 | 12 | 52 | 4 |
| 33-34 | 6.79 | 206 | 38 | 27 | -65 | 6 | 22 |

60,000 m³ of sediment eroded from the landslide dam are stored on intramontane flats within the gorge, upstream of the survey area. Small-scale pulses of sediment mobilized by flooding from this supply of stored sediment may be periodically deposited on the fan at the mouth of the gorge, complicating the overall post-landslide reduction in sediment supply and floodplain lowering.

In summary, most of surveyed reach of Poerua River aggraded from 1999 until 2005, but the river began to incise its floodplain in the following three years. A similar pattern of localized aggradation followed by degradation is shown by the volume data.

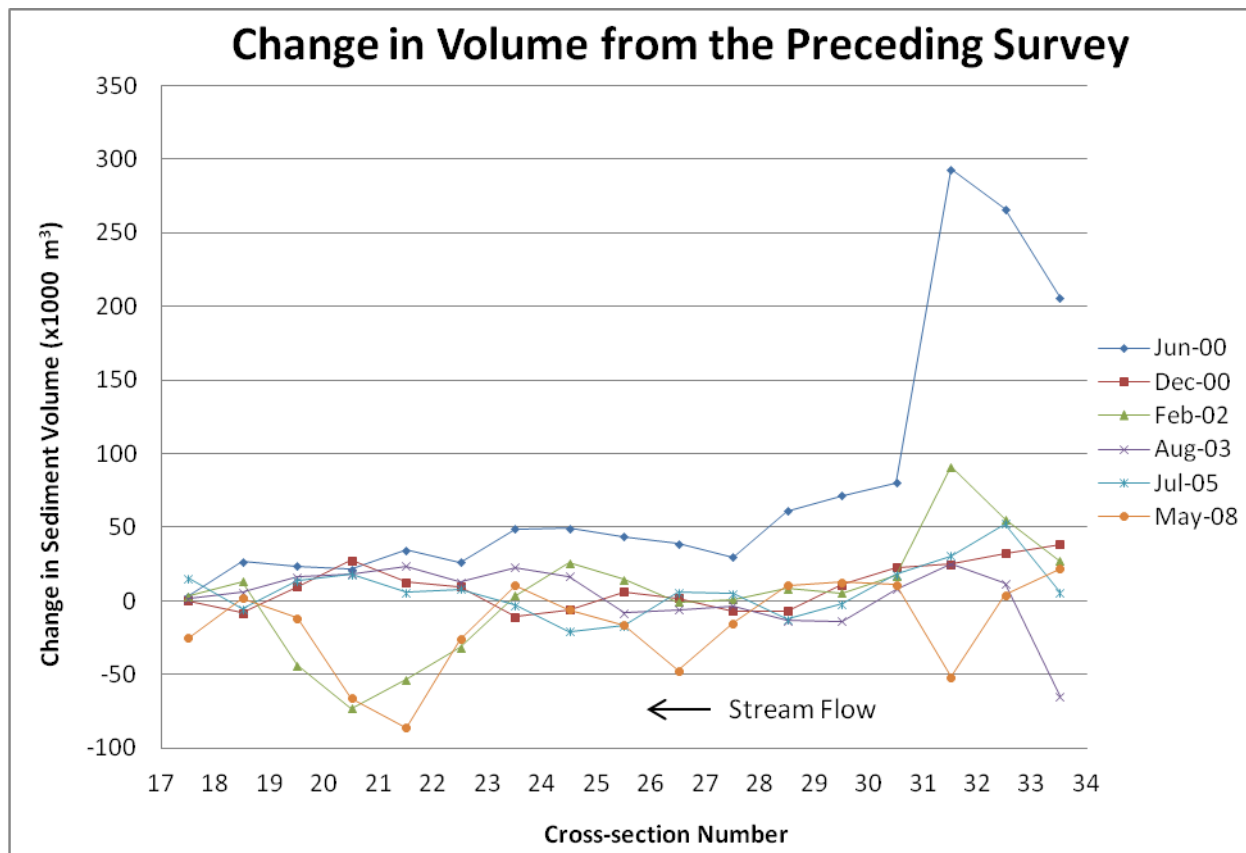


Figure 3.7. Volume changes at cross-sections 17-34 between successive surveys. See Figure 3.1 for location of sections and Table 3.1 for distances of sections of SH6 bridge.

Although the survey revealed a dominant incision trend along the lower Poerua River between the 2005 and 2008, there is a longer temporal pattern of aggradation and degradation since the landslide and dam failure. Comparison of the 2002 and 2005 survey data reveal six cross-sections with an inferred decrease in sediment volume, whereas ten cross-sections record a decrease in volume between 2005 and 2008. Comparison of the 2005 and 2008 surveys, however, does not show a consistent spatial pattern of aggradation and degradation (Table 3.5). This fluctuating pattern is similar to the pattern of wave-like movements displayed by the channel bed level data.

3.4.3 Sediment Discharge

The total volume of sediment deposited on the Poerua River fan (cross-sections 17-34) from February 2002 to July 2005 is ~ 169,000 m³. The average sediment volume over this approximately 3.5-year period is about 49,500 m³ a⁻¹ (Table 3.6). In the following three years, from July 2005 to May 2008, there is a net loss of sediment from the Poerua River fan of about 279,000 m³ or 98,500 m³ a⁻¹.

Table 3.6. Net sediment delivery to the Poerua River fan (cross-sections 17-34).

| Period | Months | Total sediment volume (m ³) | Average annual sediment volume (m ³ a ⁻¹) |
|-----------------------|--------|---|--|
| February 02 – July 05 | 41 | 169,000 | 49,500 |
| July 05 - May 08 | 34 | -279,000 | -98,500 |

The sediment budget for all cross-sections is similar (Table 3.7). From February 2002 to July 2005, 208,000 m³ of sediment were deposited within the survey area, which is equal to an average annual value of 61,000 m³. From July 2005 to May 2008, -378,000 m³ of sediment was lost from the total surveyed reach, equal to 134,000 m³ a⁻¹.

Table 3.7. Net sediment delivery for all 34 cross-sections.

| Period | Months | Total sediment volume (m ³) | Average annual sediment volume (m ³ a ⁻¹) |
|-----------------------|--------|---|--|
| February 02 - July 05 | 41 | 208,000 | 61,000 |
| July 05 - May 08 | 34 | -378,000 | -134,000 |

3.5 Discussion

3.5.1 Sediment Discharge and Sediment Yield

Korup et al. (2004) estimated sediment discharge rates for the first three years following the Mt. Adams landslide. They found that the average sediment discharge rate for the Poerua River watershed between May 1999 and February 2002 was about twice that of several other Westland watersheds (Table 3.8). Between 2002 and 2008, however, Poerua River sediment discharge decreased to values of neighboring watersheds (Hicks et al. 1996).

These estimates are conservative; they do not include sediment stored in the Poerua River gorge between the landslide dam and the apex of the Poerua River fan. Neither this study nor Korup et al. (2004) includes sediment deposited in the avulsion channel on the Poerua River fan from April 2001 to April 2003.

A river removes aggraded sediment following a disturbance event to re-establish equilibrium (Miller and Benda 2000). Griffiths (1979) observed channel aggradation followed by incision along Waimakariri River in New Zealand following bank failures. Madej and Ozaki (1996) documented channel aggradation, subsequent degradation, and channel widening along Redwood Creek in California after sediment supply increased due to bank erosion. As a river incises its bed, however, the channel may become armoured with coarse gravel or boulders, impeding or stopping further incision. Miller and Benda (2000) noted channel widening, braiding, and fining of bed material the introduction of sediment into the Gate Creek in Oregon by debris flows. After the sediment influx had passed they noted channel incision down to an immobile, armoured bed load. Poerua River may show a similar response, and its channel on the fan head may not reach the level it had before the dam break flood for decades or longer. Similar effects of a dam break flood in the Rocky Mountains persisted for more than a decade (Bathurst and Ashiq 1998). Morche and Schmidt (2011) found that studies of the post dam-break effects on fluvial systems must span more than a decade to definitively conclude that the system has returned to a pre-failure state.

Landslide dams pose significant hazards to settlements and farmland downstream (Davies and Scott 1997). Outburst floods from landslide-dammed lakes are an obvious hazard; less appreciated is the hazard posed by the greatly increased sediment supply following breaching of the landslide dam. When sediment input to a river reach is greater than output from the reach, the river responds by aggrading and widening its channel or by avulsing (Madej and Ozaki 1996). Later, the river incises the aggradational fill along

Table 3.8. Average sediment delivery to cross-sections 17-34 (modified from Korup et al. 2004).

| Period | Duration (months) | Contributing watershed area (km ²) | Sediment volume input (m ³) | Apparent sediment discharge (m ³ a ⁻¹) ^a | Specific sediment yield (t km ⁻² a ⁻¹) ^b | Source |
|---------------------------|-------------------|--|---|--|--|---------------------------------|
| May 99 - November 99 | 6 | 59 | 932,800 | 1,865,600 | 57,000 | Korup et al. 2004 This study |
| November 99 - June 00 | 7 | 59 | 342,700 | 587,500 | 18,000 | Korup et al. 2004 This study |
| June 00 - December 00 | 6 | 59 | 165,500 | 330,900 | 10,000 | Korup et al, 2004 This study |
| December 00 - February 02 | 14 | 59 | 163,400 | 140,100 | 4300 | Korup et al. 2004 This study |
| February 02 - August 03 | 18 | 59 | 52,700 | 35,200 | 1100 | This study |
| August 03 - July 05 | 23 | 59 | 116,600 | 60,800 | 1900 | This study |
| July 05 - May 08 | 34 | 59 | -279,000 | -98,500 | -3,000 | This study |
| May 99 - May 08 | 108 | 59 | 1,438,200 | 159,800 | 4,900 | This study |

^a Not corrected for trap efficiency.

^b Bulk density assumed at 1.8 t m⁻³.

its steepened channel (Schuster 2006). Poerua River responded in this manner after the Mt. Adams landslide. This study shows that the river is no longer aggrading over much of the surveyed reach and is approaching a new equilibrium.

3.5.2 Error and uncertainty

I acknowledge several assumptions and sources of error. First, measurements taken with the TopCon dGPS instrument have possible errors. A range of 0.0003 m up to 1.2 m in uncertainty, although the 1.2 m error was an outlier and was discarded from the

data set. In addition, I assumed that each measurement was recorded with the utmost accuracy. Second, I assumed that the volume of sediment lost or gained between adjacent cross-sections is uniform. Thickness values used to calculate volumes were averages of those determined at the two closest cross-sections. Third, I do not take into account sediment that was deposited in the avulsion channel. The volume of sediment in this channel could amount to as much as 15% of the total sediment volume. Fourth, sediment volume estimates were not made for the entire active channel, from margin to margin. My estimate of sediment that not been accounted for due to incomplete surveying is 23%. Fifth, the volume estimates are entirely changes in sediment bedload. Finally, I did not perform the cross-section surveys prior to 2008. I assume that the data collected between 1999 and 2005 are accurate.

These sources of uncertainty and error could affect my results and conclusions made. Measurement errors, either my own or those of previous surveys, would affect the bed levels and volume estimates. Considering that some of the changes are as small as several centimeters, some of my statements regarding incision and aggradation could be compromised. Only at the apex of the fan, where the changes are on a meter scale, are my conclusions unassailable. Any error in the survey measurements would have an effect on the volume estimates. At any rate, the sediment volume estimates are conservative because the entire stream reach affected by the outburst flood was not included in the volume calculations

3.5.3 Threat to the SH6 Bridge

Hancox et al. (2005) reported only a minor (<0.5 m) build-up of sediment at the SH6 bridge following the 1999 Mt. Adams landslide. They stated that the threat to the

bridge would increase as the sediment pulse moved downstream. My study, however, shows that the mean bed elevation adjacent to the bridge has been decreasing since 2002, with a total lowering of 0.63 m since then. Thus there appears to be no threat to the bridge from aggradation. However, although the channel is incising near the SH6 bridge, the average level of the channel is still higher than it was before the landslide. With a reduced freeboard, the bridge is more vulnerable to damage or destruction during a major flood.

3.6 Conclusion

The debris dam emplaced during the 1999 Mt. Adams rock avalanche had a major impact on Poerua River. I supplemented previous topographic surveys with dGPS measurements taken in June 2008 to track the downstream redistribution of sediment from the debris dam and the return of Poerua River to equilibrium. I estimated total sediment discharge and sediment yield at each of 34 surveyed cross-sections. Data collected over the period of a decade show aggradation occurring widely up to six years after the dam-break event, but incision becoming dominant three years later. In addition, the sediment flux in the first two years after dam breach was high, but it returned to values similar to those in adjacent watersheds nine years later. Within this overall pattern of aggradation followed by degradation are smaller-scale, wave-like movements of sediment over shorter distances within the surveyed reach. A decade is too short to fully document the return of a river to equilibrium following a dam-break event.

Future repeat surveying of the cross-sections will allow the continuing reduction in sediment supply below the landslide barrier to be documented. Because the cross-

sections were first surveyed shortly before the landslide, continued surveying offers a singular opportunity to examine the impact of a large landslide on a river.

CHAPTER 4: DISCUSSION AND CONCLUSION

Both of the study areas experienced disturbances from mass-wasting events, but the disturbances differ in magnitude and character. The Waitangitaona River watershed was affected by the Gaunt Creek slip. This landslide increased sediment delivery to the lower reaches of the river and triggered an avulsion of the river channel and aggradation of the floodplain. After 40 years, the lower Waitangitaona River fan is still aggrading in response to the landslide. Poerua River was dammed by a rock avalanche from Mt. Adams in 1999. Failure of the landslide dam delivered large amounts of sediment to the fan at the west front of the Southern Alps, causing rapid aggradation (up to 4 m) over the next six years. Subsequently, the river began to incise the fan as the supply of sediment from the landslide dam diminished.

Sediment yield also differs in the two watersheds. The average sediment yield in the Waitangitaona River watershed over the four-decade period of my study is $30,000 \text{ t km}^{-2} \text{ a}^{-1}$. The sediment yield in the Poerua River watershed, largely associated with breaching of the Mt. Adams landslide dam, reached a maximum of $57,000 \text{ t km}^{-2} \text{ a}^{-1}$ between May 1999 and November 1999, but decreased to $1900 \text{ t km}^{-2} \text{ a}^{-1}$ by July 2005 (the average sediment yield from May 1999 to May 2008 is $4900 \text{ t km}^{-2} \text{ a}^{-1}$). The difference in annual sediment yield between the two study areas is almost an order of magnitude, which is unusual considering that the two watersheds have the same physiographic, geologic, and tectonic settings. Sklar and Dietrich (1998 2004) noted that landslide dams greatly impact sediment flux along a stream because upstream sediment

is trapped in natural reservoirs. The reduction in sediment supply commonly leads to incision of the stream channel downstream of the dam.

4.1 Sediment Yield in Other Regions

Sediment yield differs greatly throughout the world, as well as through time. For example, sediment yields range from 740 to 5300 t km⁻² a⁻¹ in southern coastal California (Warrick et al. 2009), 766 to 933 t km⁻² a⁻¹ in the Ecuadorian Andes (Laraque et al. 2004), and 355 to 1197 t km⁻² a⁻¹ in the Himalayas (Ali and De Boer 2005). In comparison, sediment yields for South Westland watersheds are 4 to 10 times these values. These extraordinarily high values are the result of high annual precipitation, rapid uplift, weak bedrock, and episodic large earthquakes. Sediment yields in South Westland watersheds are two orders of magnitude larger than in Fiordland, farther south on the South Island of New Zealand. The probable cause is the lower mean local relief and the presence of stable landslide dams in the latter area (Korup 2005). Sediment yields differ considerably between neighbouring watersheds in South Westland because of the different characteristics and magnitudes of mass-wasting events.

4.2 Effects over Time

Several studies of outburst floods resulting from failures of landslide dams have shown that sediment discharge is highest just after the dam break event and decrease over time (Costa 1985; Schuster 2006). The Poerua River dam break event displayed this pattern. The highest sediment discharge was directly after the dam failure and, over a period of about six years, decreased to pre-disturbance values. Poerua River incised its fan, and the dam continued to be eroded until at least the time of my study. Morche and

Schmidt (2011) documented a similar timescale for reestablishment of background sediment yields following a dam break event on Partnach River in the German Alps.

Landslides play an important role in sediment supply in South Westland, consistent with the finding of Keefer (1994) that landslides are the dominant agent of long-term erosion in seismically active regions. The Gaunt Creek slip contributed about 39% of the total sediment from the Waitangitaona River watershed over the 41-year period between 1967 and 2008 and continues to contribute sediment to the system today.

The characteristics of landslide events determine whether an elevated supply of sediment to the fluvial system will persist over a long period (Waitangitaona River) or simply represent a short-term perturbation of the normal background condition (Poerua River). The lower Poerua River experienced an elevated supply of sediment for a short period of time. The sediment pulse caused rapid aggradation, channel avulsion, changes to river planform, and burial of riparian vegetation. The system began to return to an equilibrium state, however, within years of the landslide that perturbed the system. Poerua River continues to adjust to the disturbance in 1999, but the changes are much smaller now than in the years immediately following the landslide

In contrast, the lower Waitangitaona River is experiencing the effects of elevated sediment supply from a landslide 40 years after the event. Sediment from the landslide has had similar effects to the outburst flood on Poerua River, although aggradation rates on the Waitangitaona River fan are much lower (0.09 m a^{-1}) than on the Poerua River fan (maximum of 0.66 m a^{-1}) and the Waitangitaona fan is still experiencing aggradation whereas the Poerua fan is now degrading.

4.3 Limitations and Suggestions for Future Studies

This project has limitations stemming from the methods used in the two study areas. Ground-penetrating radar (GPR) was the primary tool used in the Waitangitona study to estimate sediment yield. Additional GPR profile lines, especially in the areas that I did not survey, would provide better estimates of the thickness of sediment deposited since 1967. In addition, this study could have been enhanced by construction of a high-resolution digital elevation model and a more detailed investigation of the Lake Wahapo delta. A bathymetric survey of the lake and acquisition of sediment cores would further constrain the sediment yield estimates.

The Poerua River study was based on differential GPS measurements of the level of the river bed. Future repeat surveys of the same cross-sections would extend the record of channel adjustments to the Mt. Adams landslide in 1999. In addition, examination of historic aerial photographs, creation of DEMs, documentation of grain size distributions of sediment deposited following the dam breach, and a survey of channel gradient would provide further insight into the nature of the changes to Poerua River since 1999.

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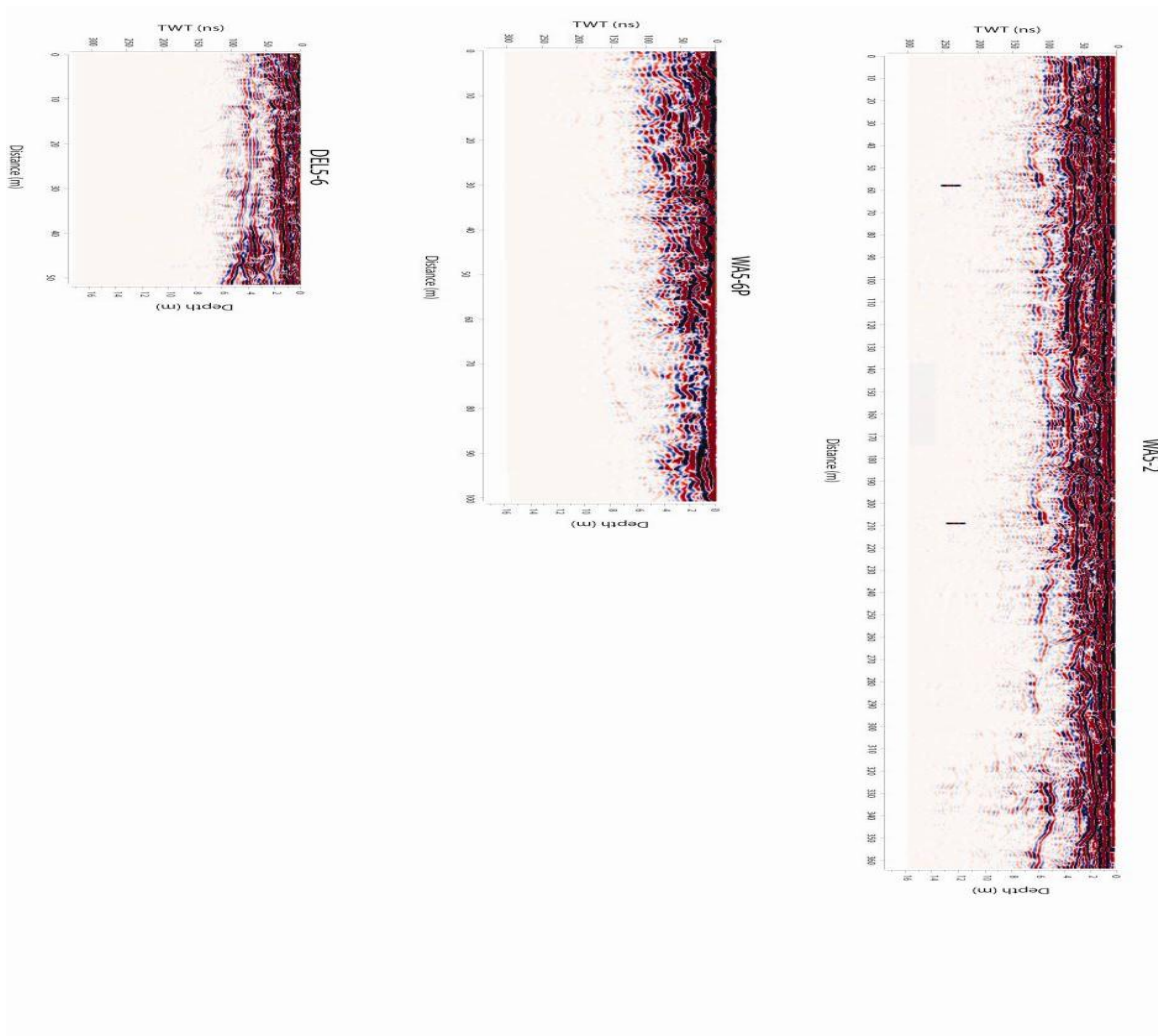
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APPENDICES

Appendix A: GPR profiles.

Figure 1. GPR profiles for lines WA5-2, WA5-6P, and DEL5-6.



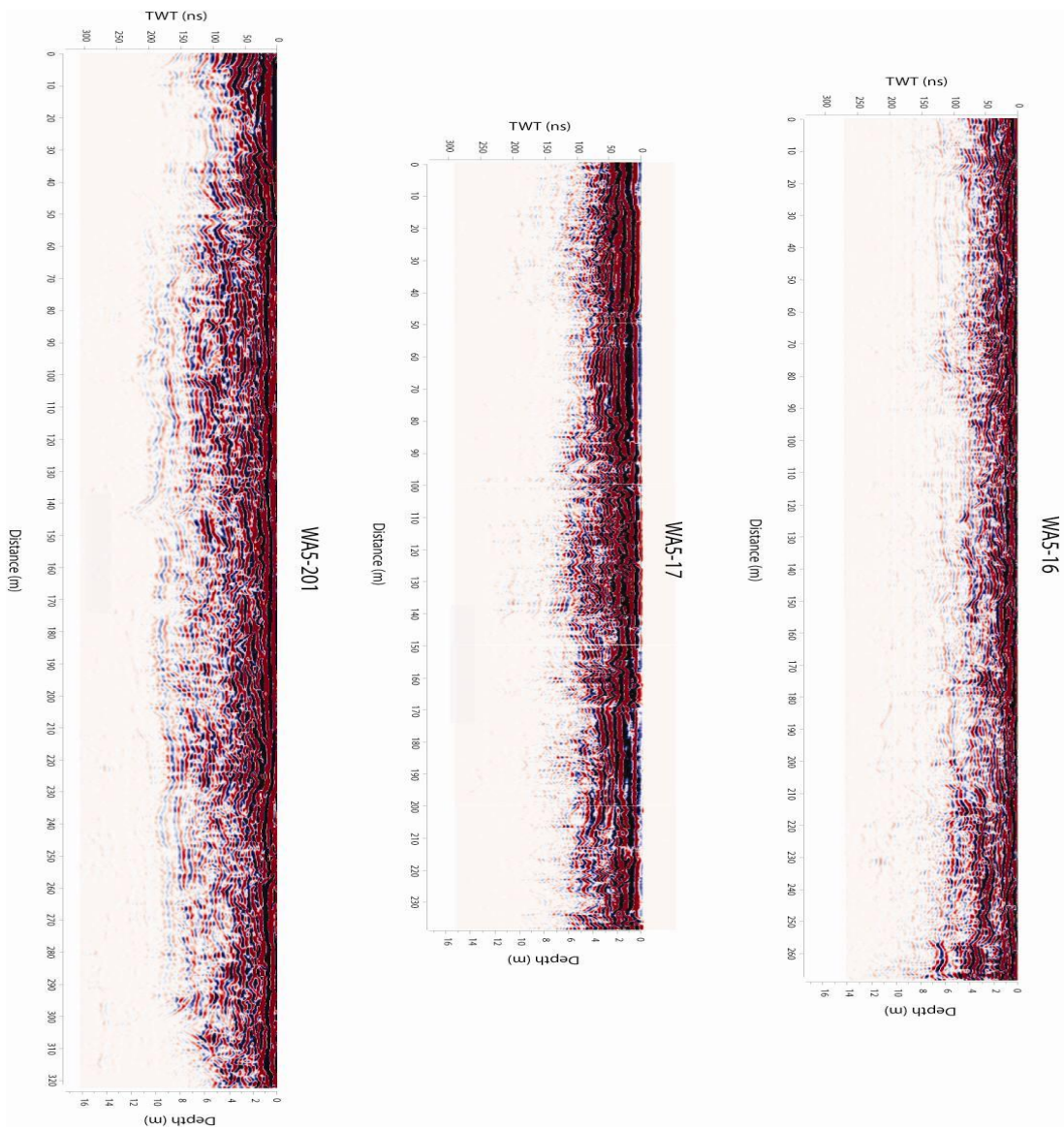


Figure 2. GPR profiles for lines WA5-16, WA5-17, and WA5-201.

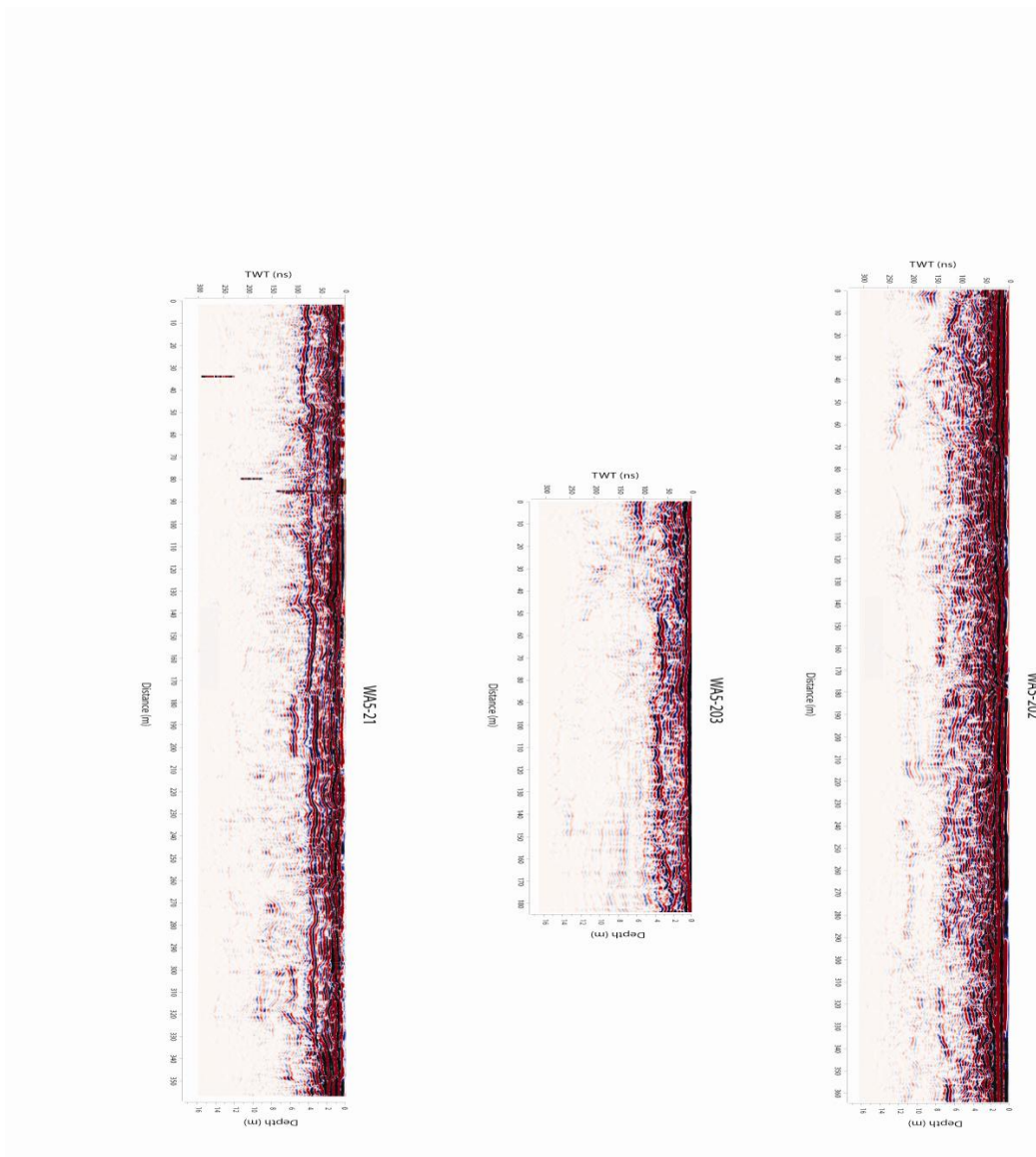


Figure 3. GPR profiles for lines WA5-202, WA5-203, and WA5-21.

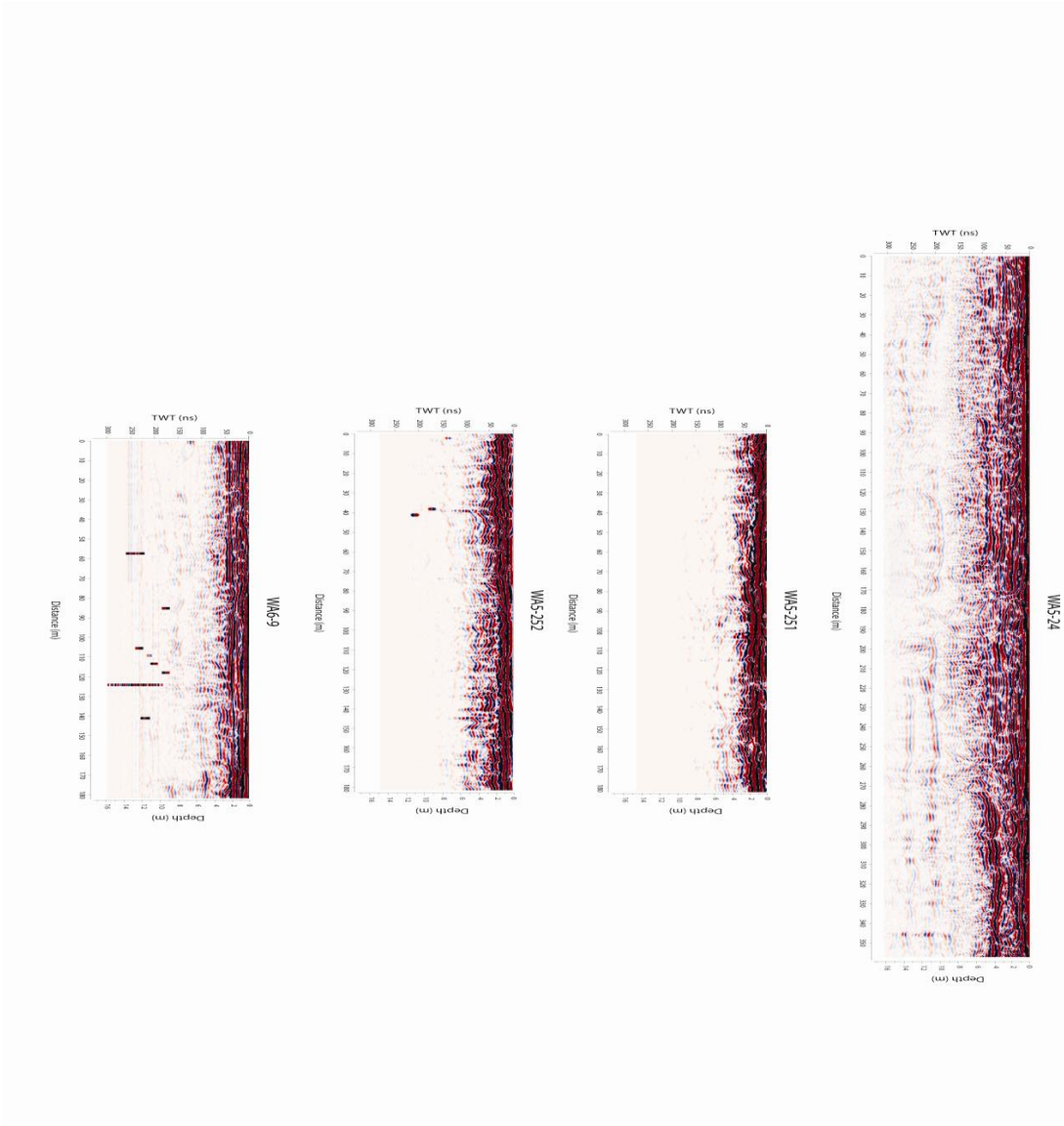


Figure 4. GPR profiles for lines WA5-24, WA5-251, WA5-252, and WA6-9.

Appendix B: Bed levels for each cross-section.

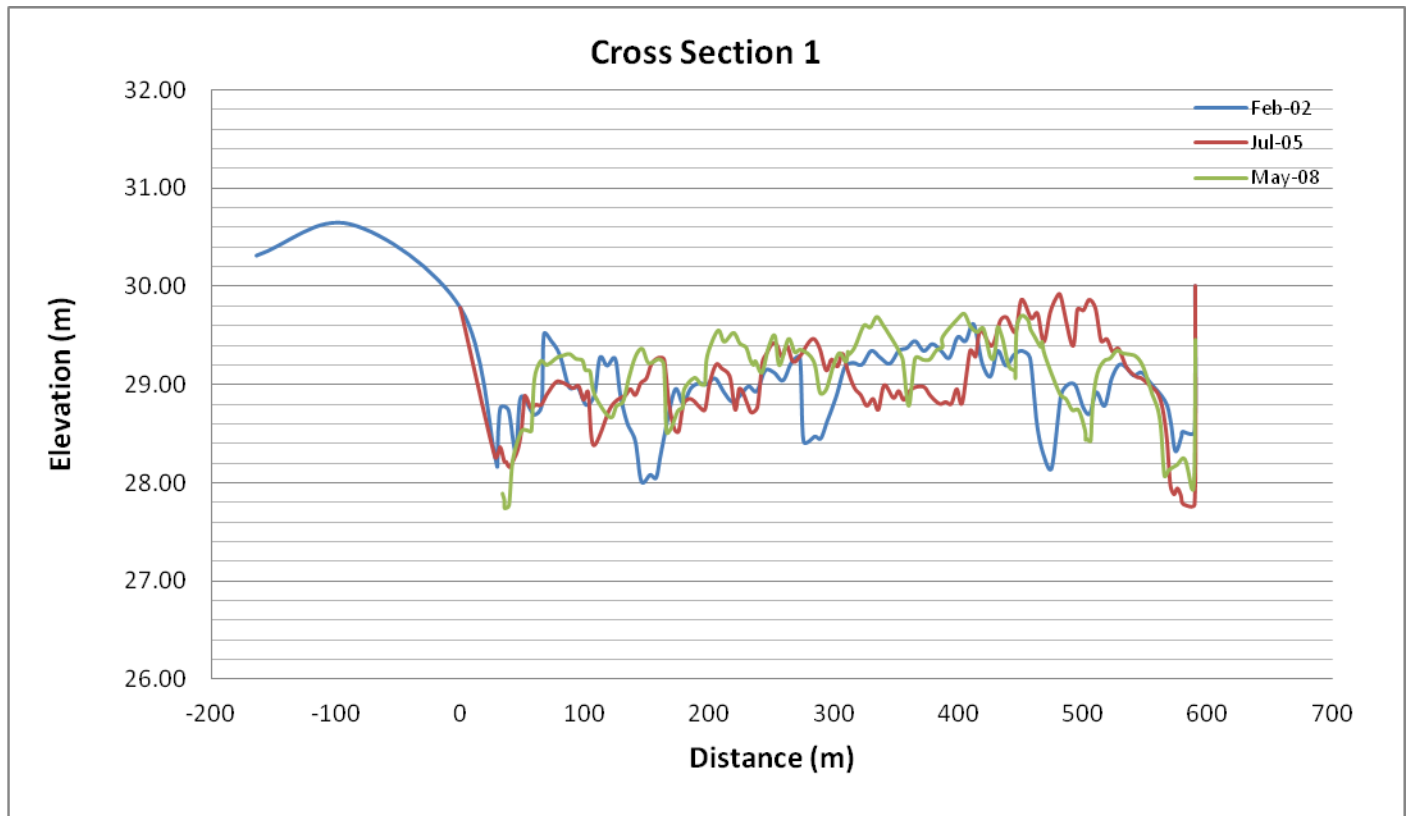


Figure 1. Bed levels at cross-section 1 on February 2002, July 2005, and May 2008.

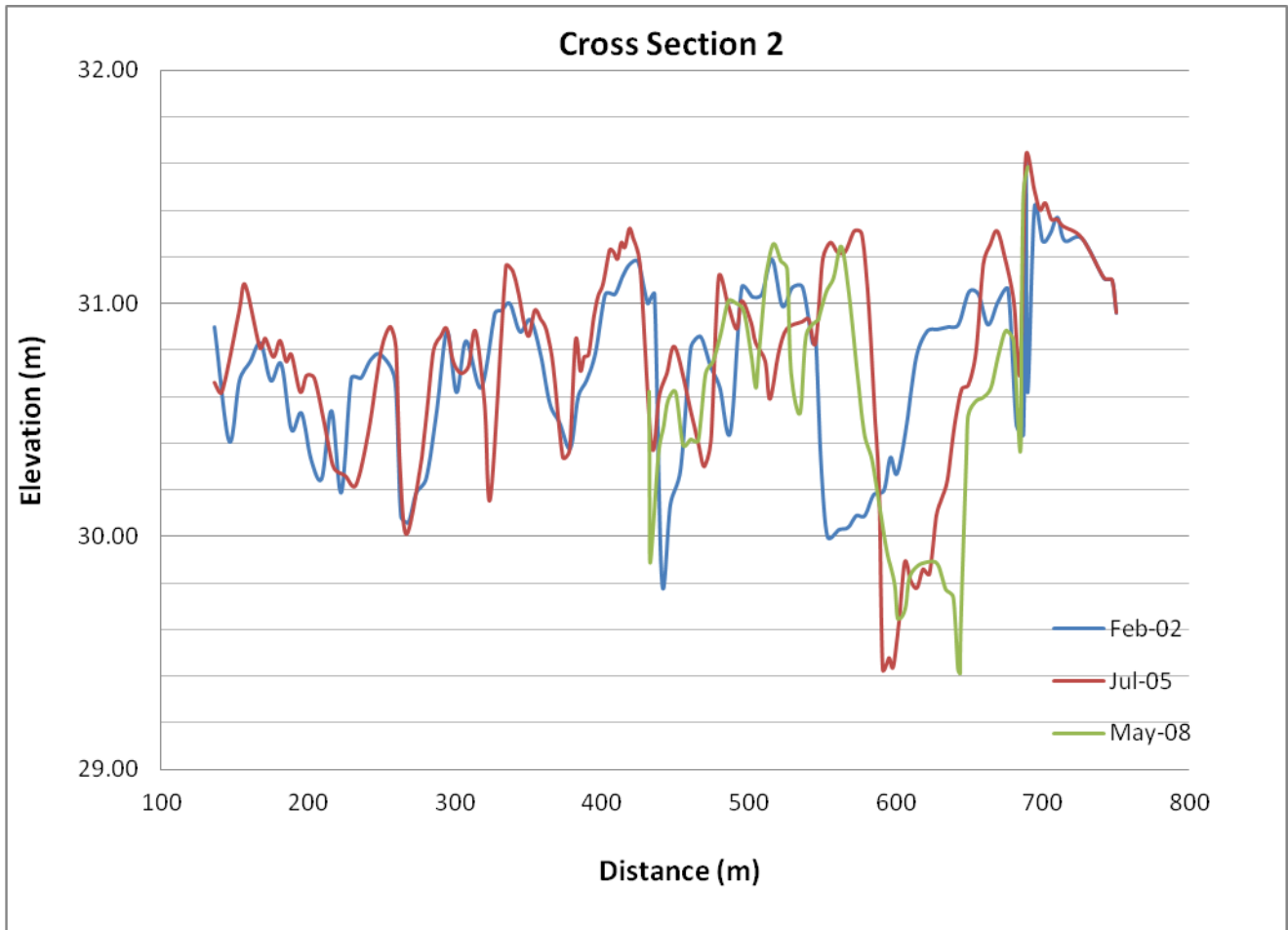


Figure 2. Bed levels at cross-section 2 on February 2002, July 2005, and May 2008.

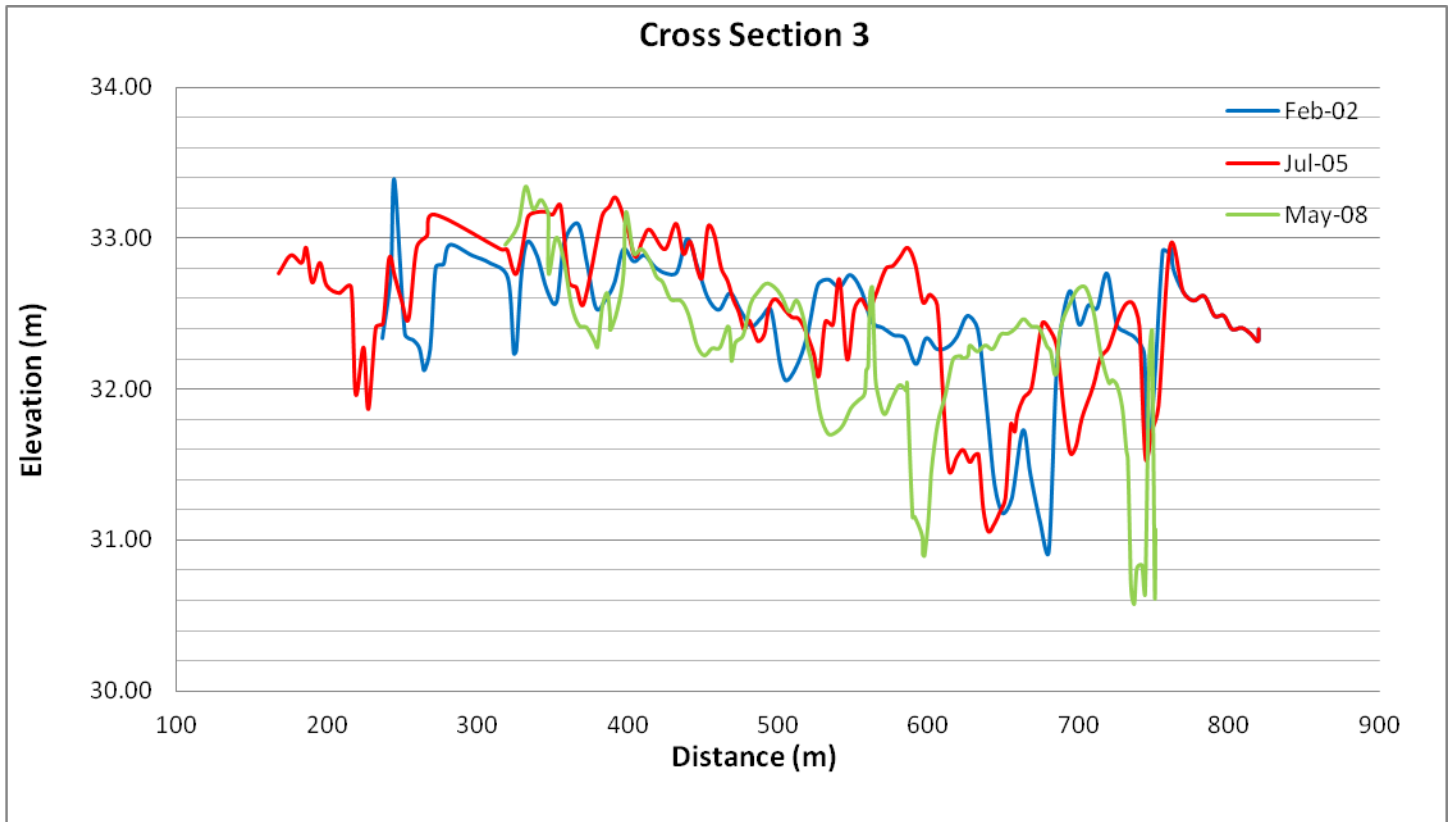


Figure 3. Bed levels at cross-section 3 on February 2002, July 2005, and May 2008.

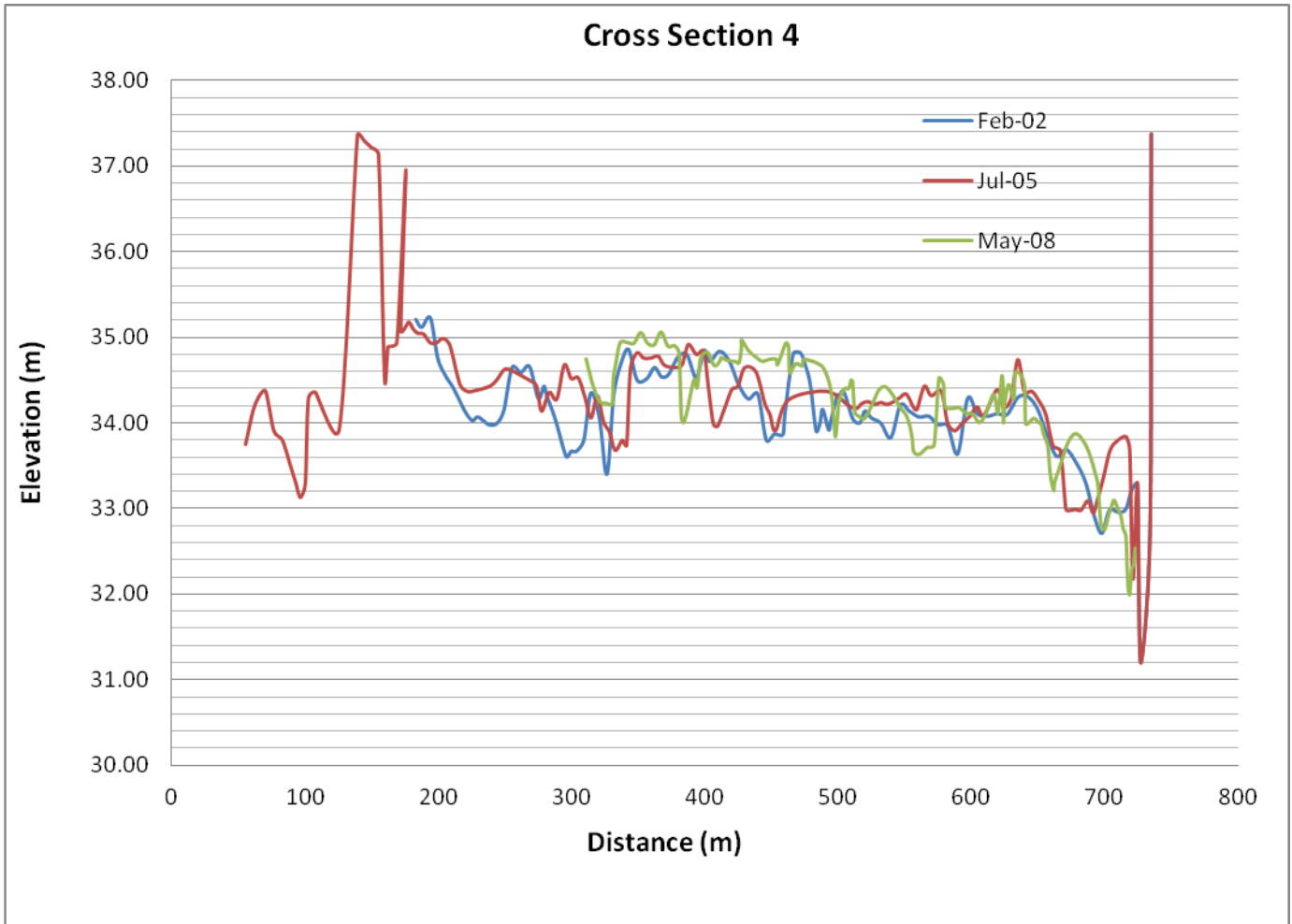


Figure 4. Bed levels at cross-section 4 on February 2002, July 2005, and May 2008.

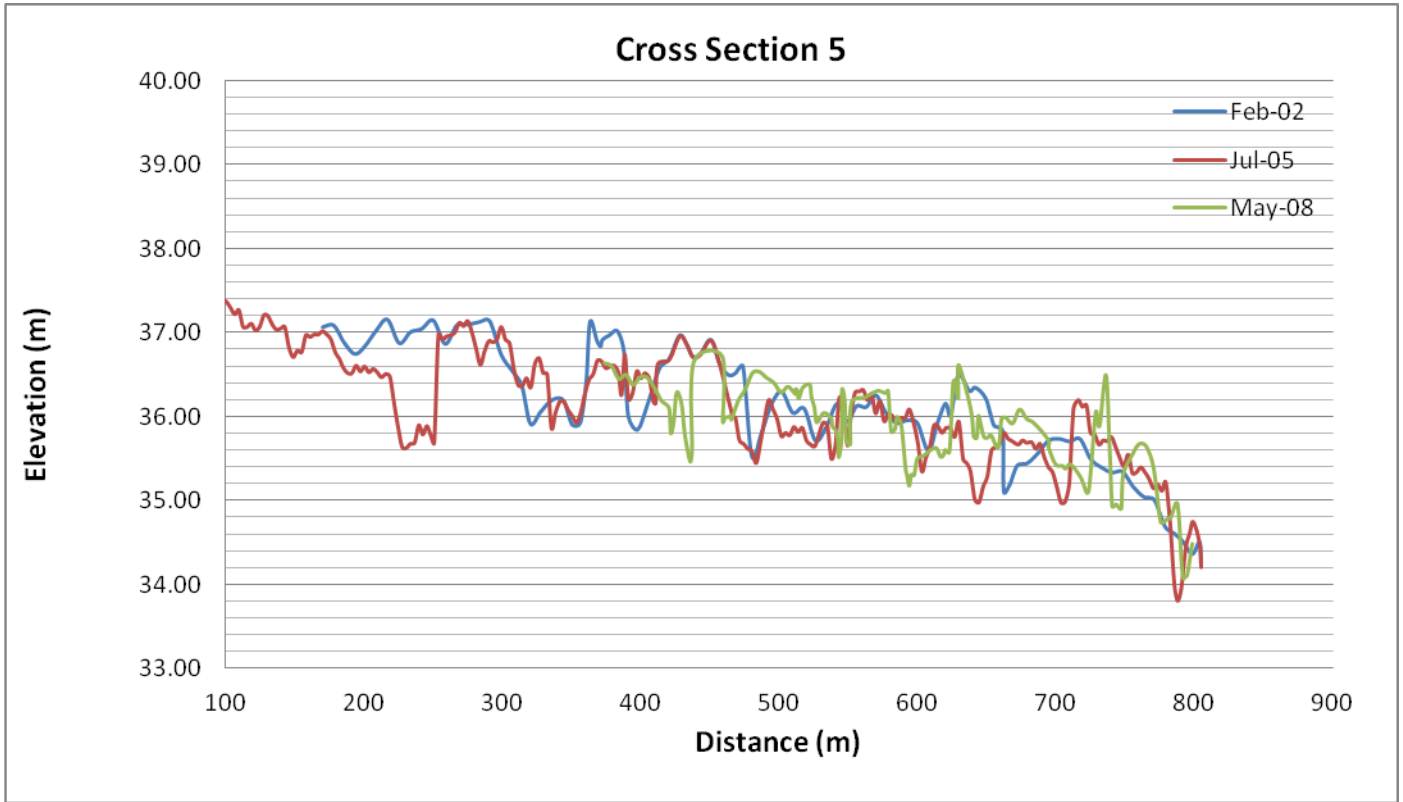


Figure 5. Bed levels at cross-section 5 on February 2002, July 2005, and May 2008.

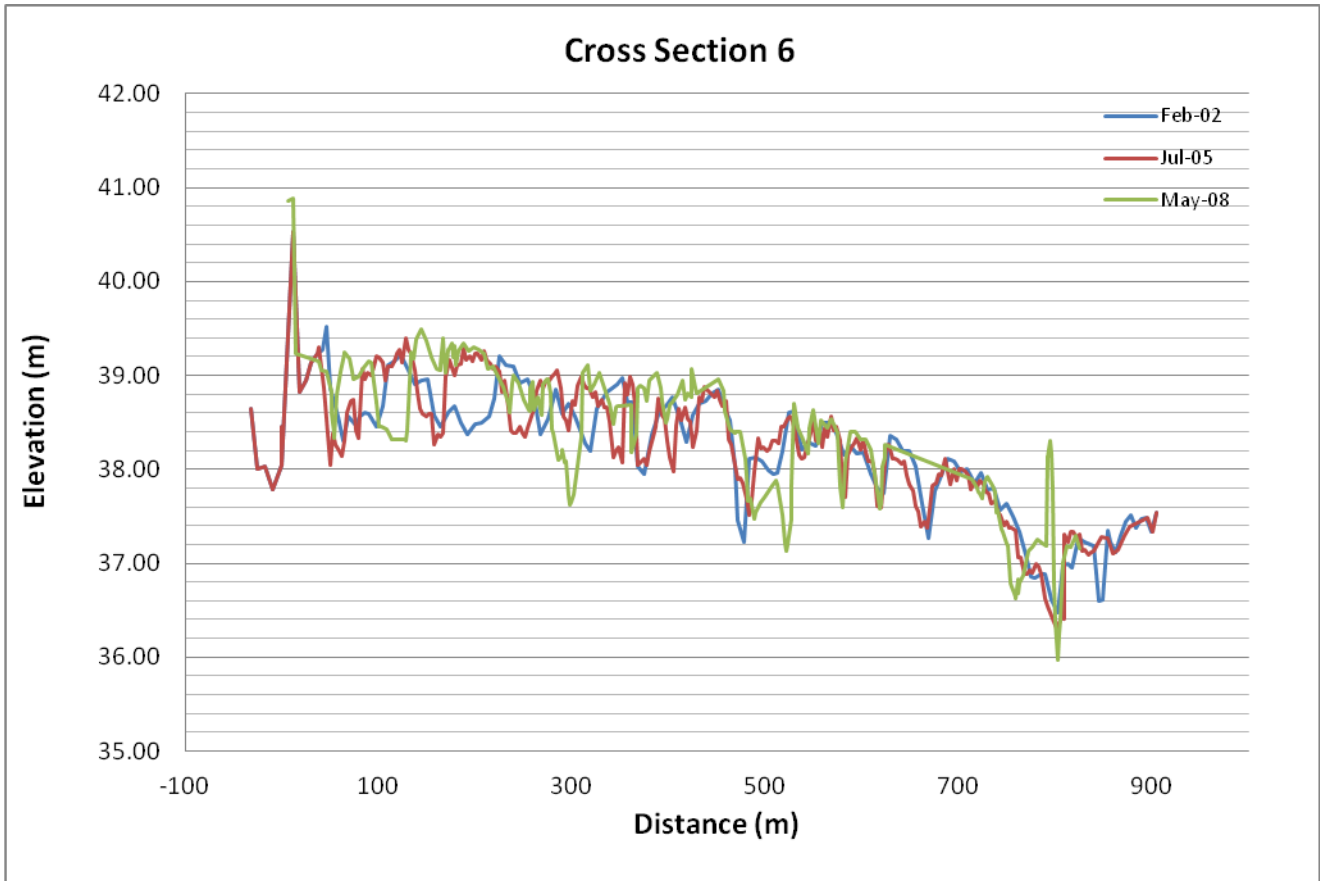


Figure 6. Bed levels at cross-section 6 on February 2002, July 2005, and May 2008.

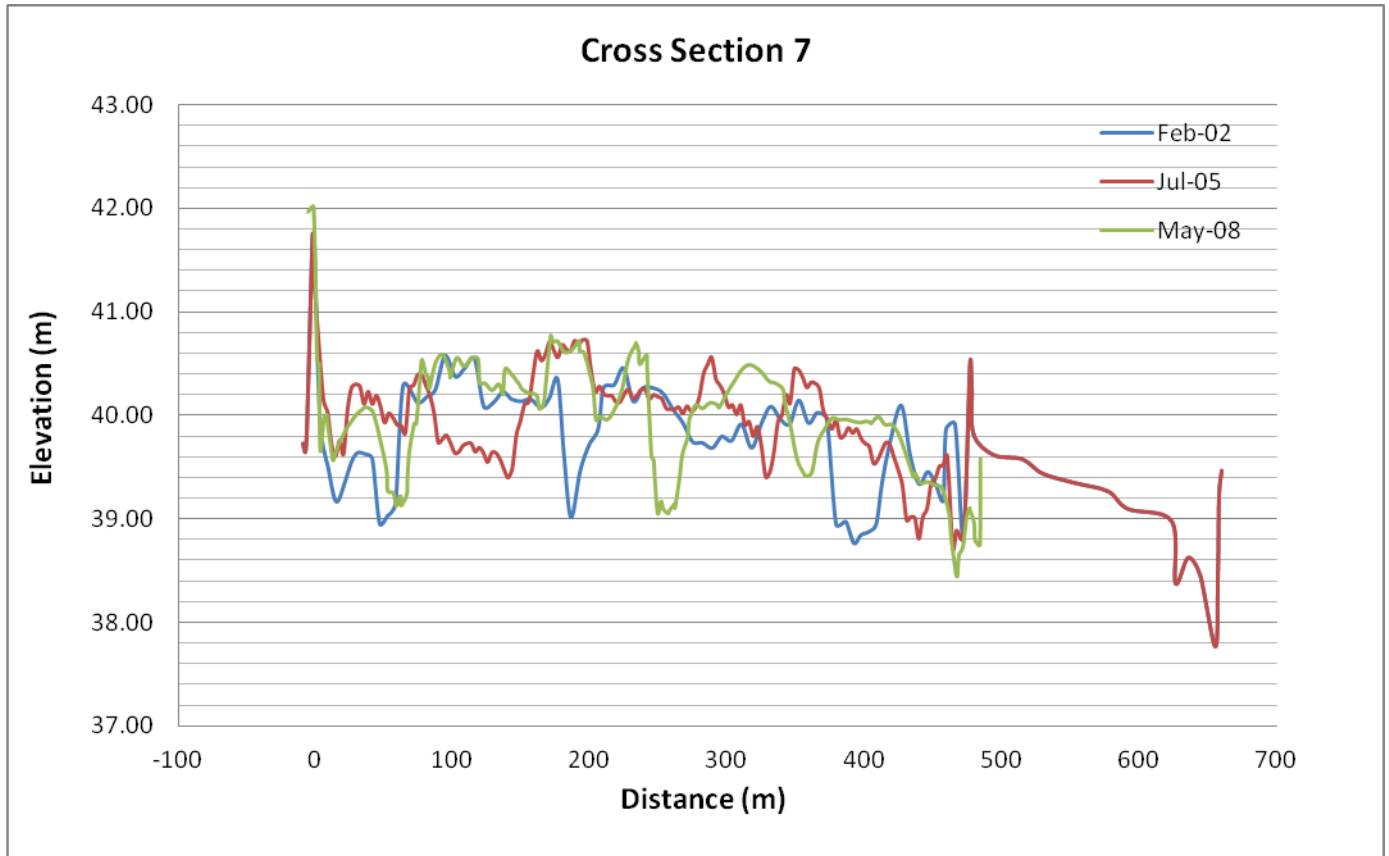


Figure 7. Bed levels at cross-section 7 on February 2002, July 2005, and May 2008.

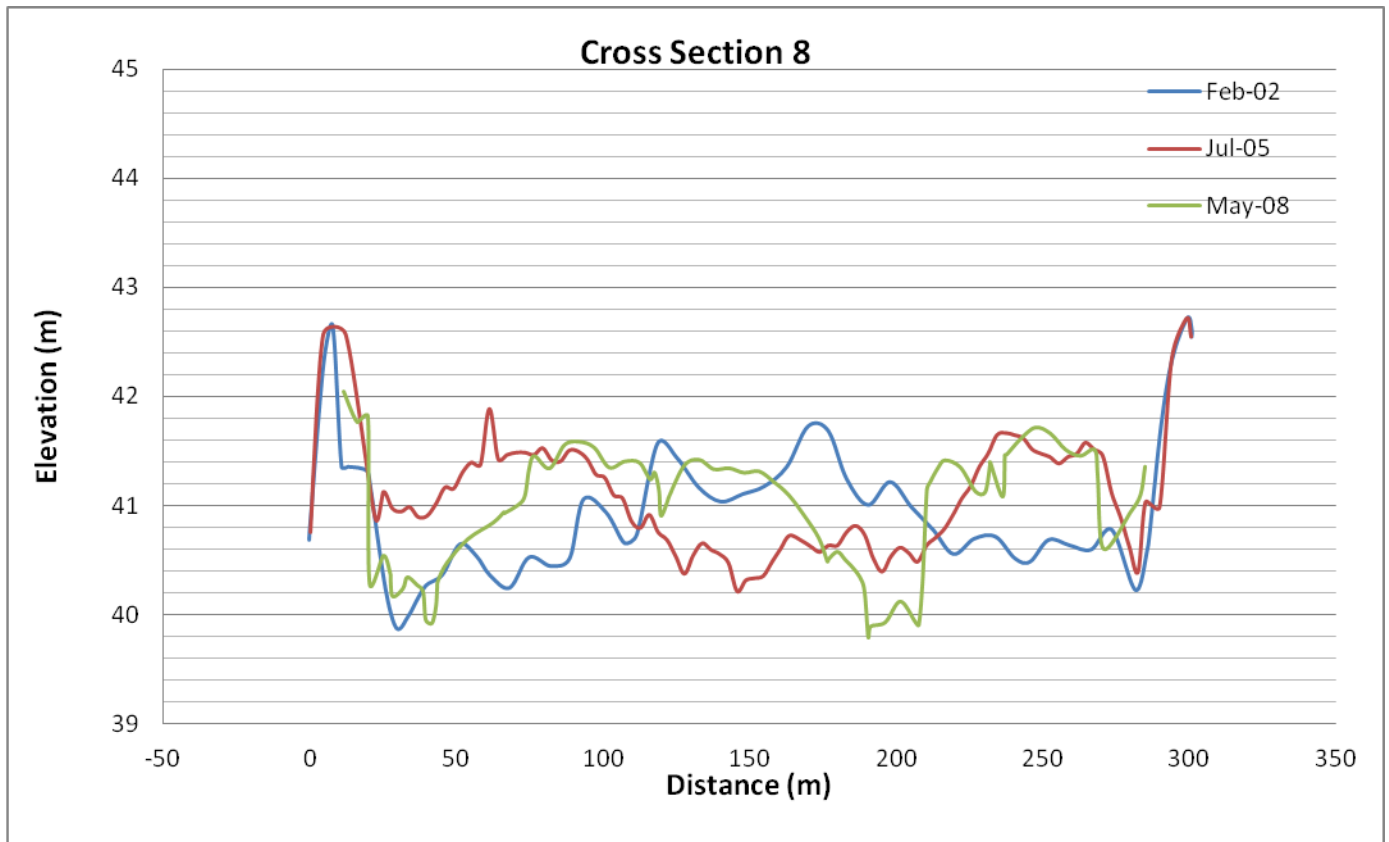


Figure 8. Bed levels at cross-section 8 on February 2002, July 2005, and May 2008.

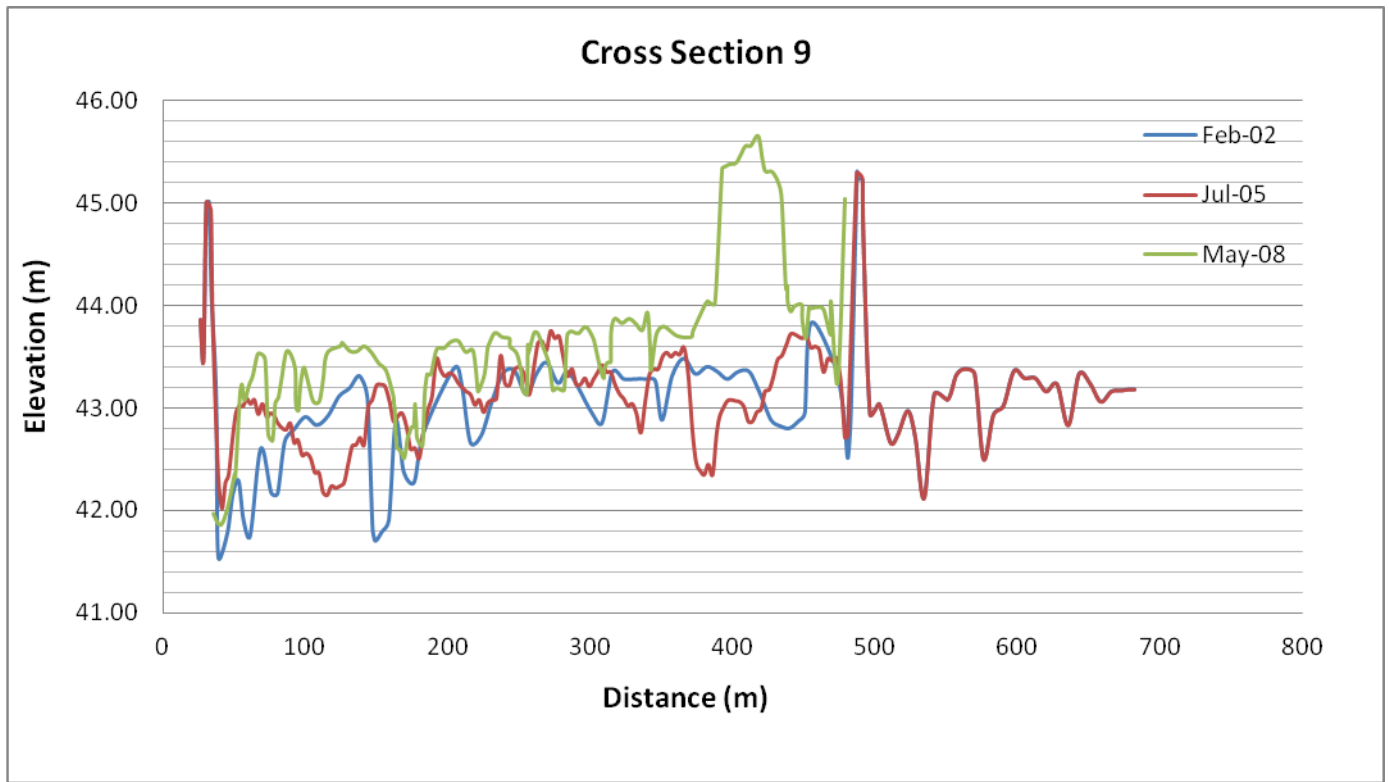


Figure 9. Bed levels at cross-section 9 on February 2002, July 2005, and May 2008.

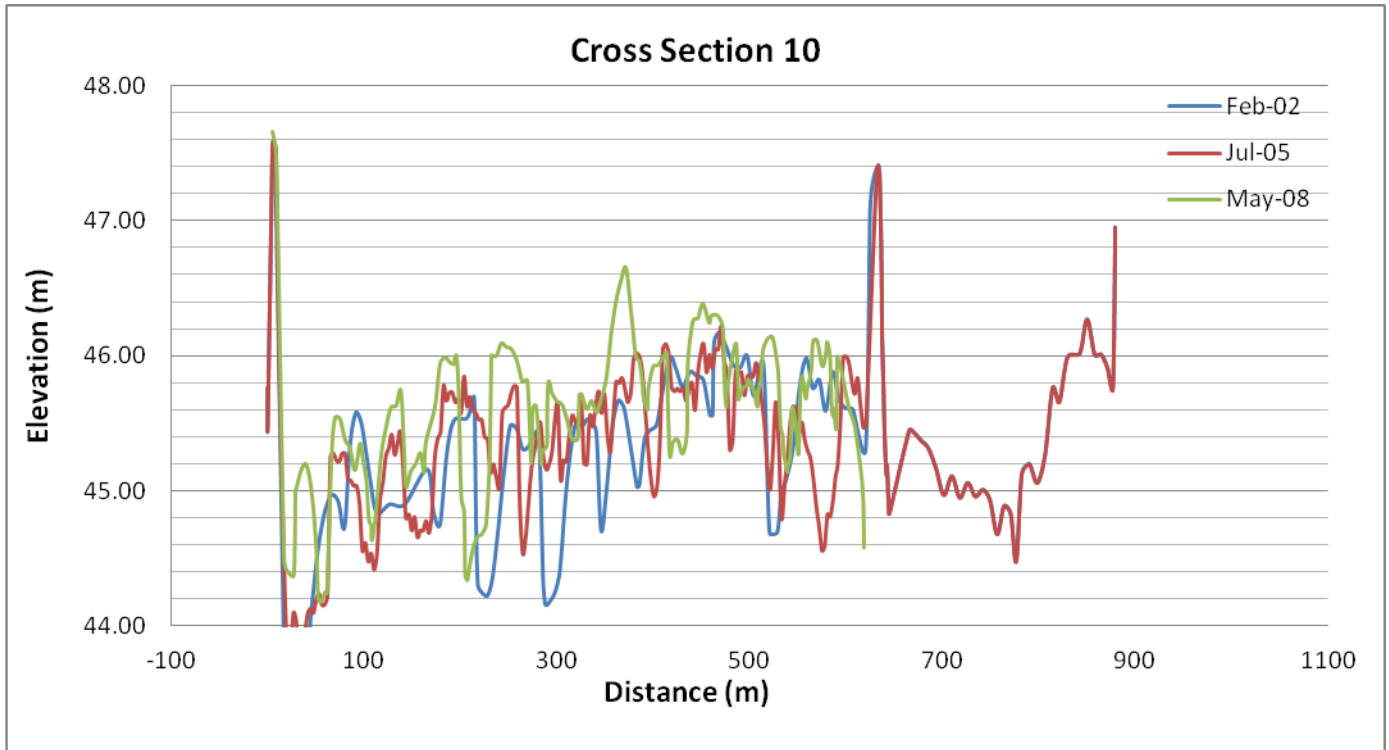


Figure 10. Bed levels at cross-section 10 on February 2002, July 2005, and May 2008.

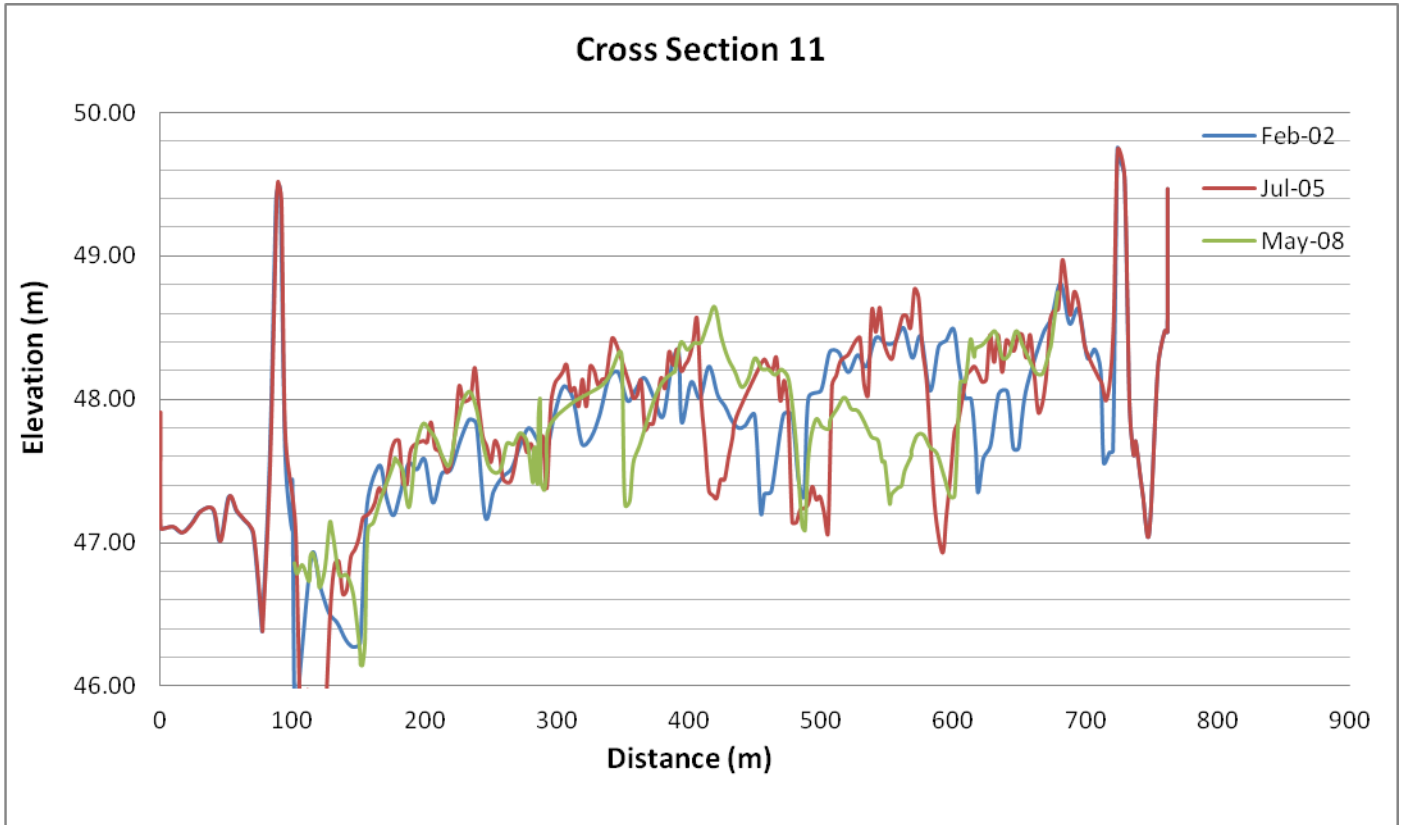


Figure 11. Bed levels at cross-section 11 on February 2002, July 2005, and May 2008.

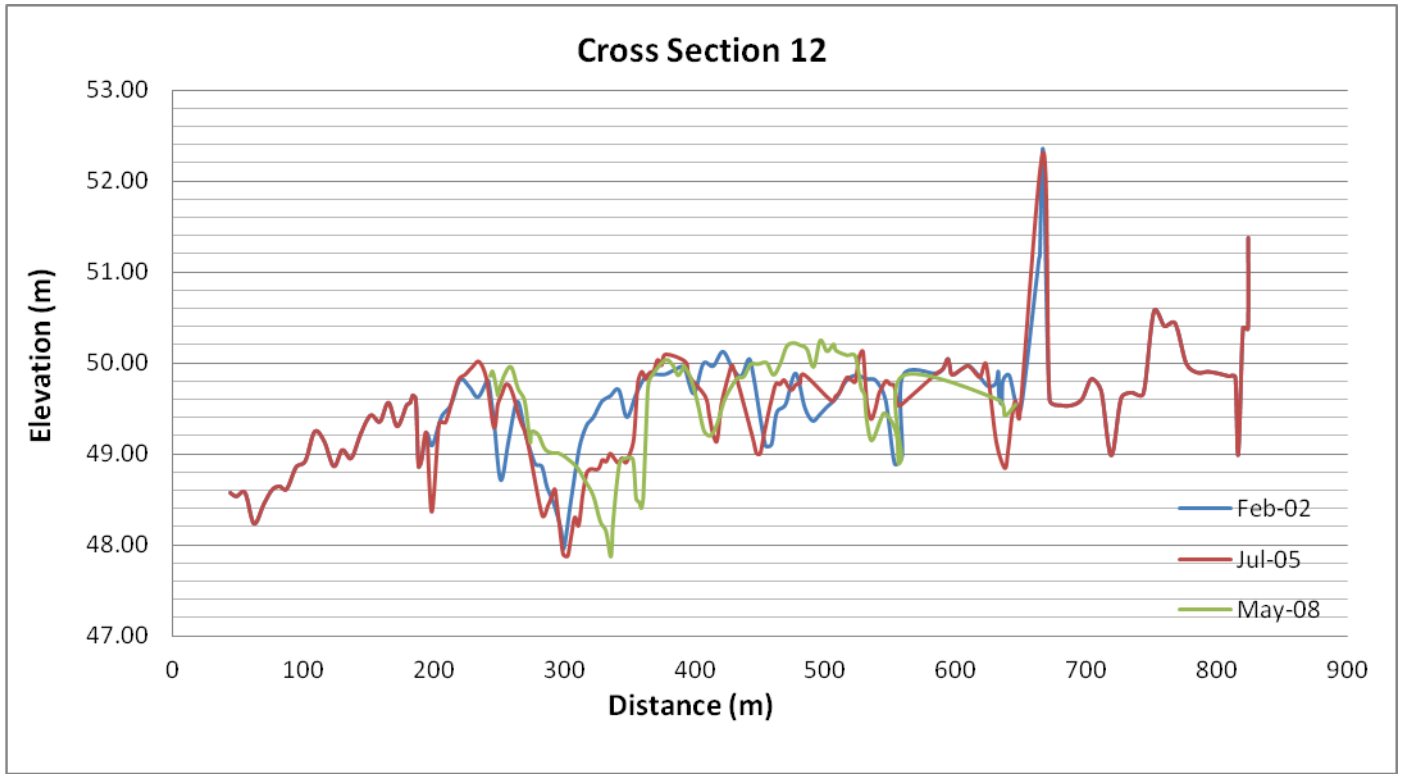


Figure 12. Bed levels at cross-section 12 on February 2002, July 2005, and May 2008.

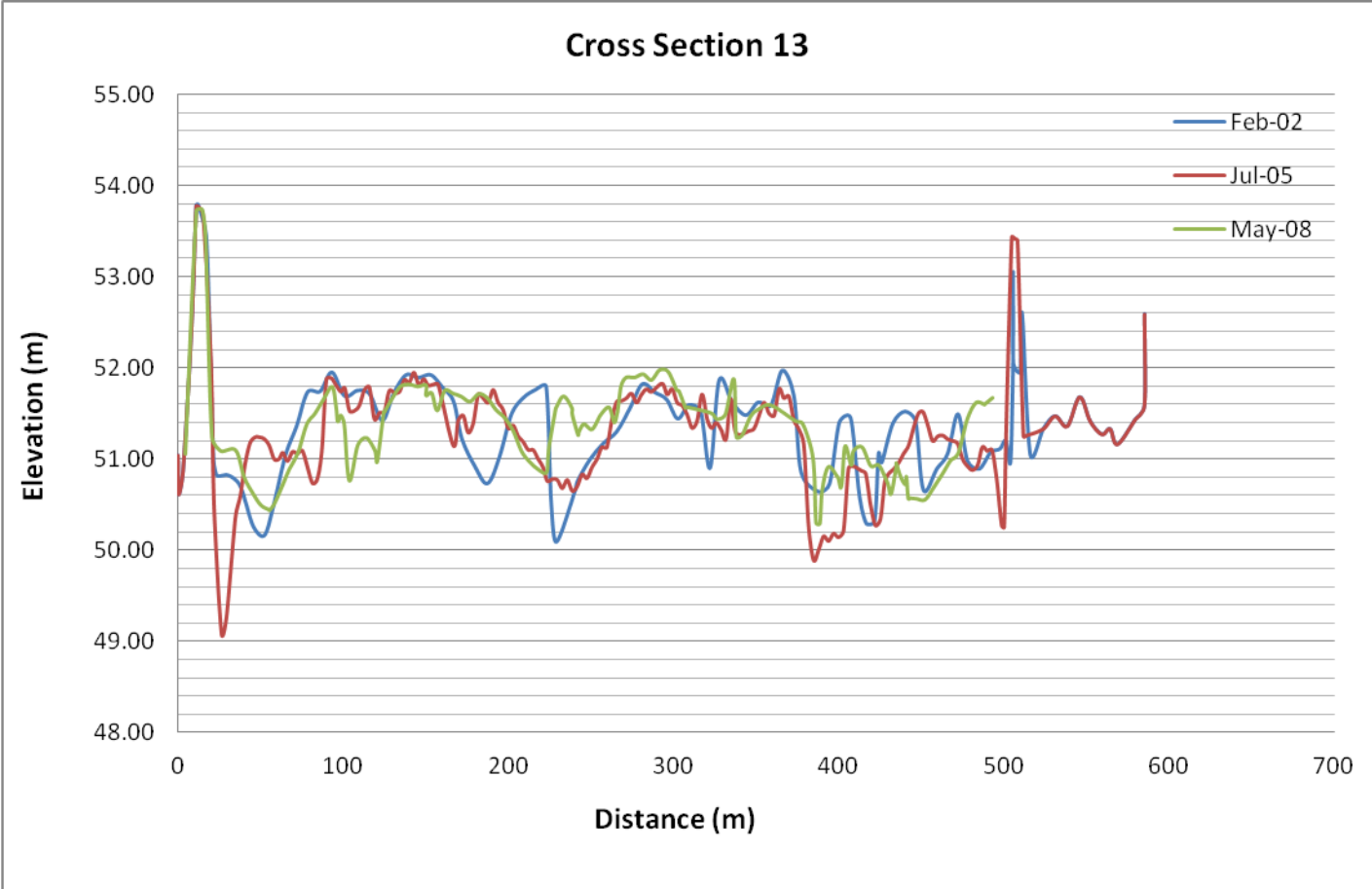


Figure 13. Bed levels at cross-section 13 on February 2002, July 2005, and May 2008.

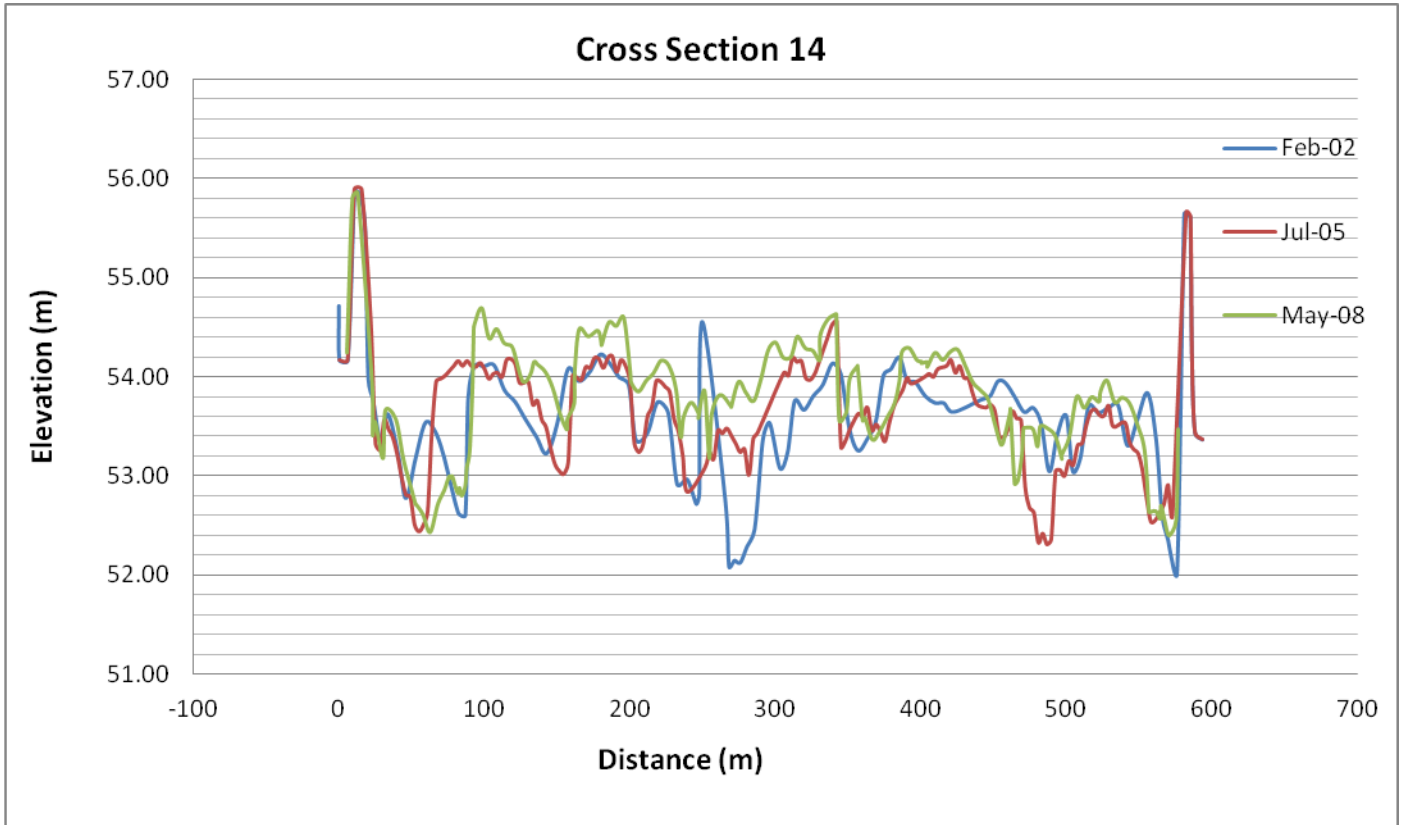


Figure 14. Bed levels at cross-section 14 on February 2002, July 2005, and May 2008.

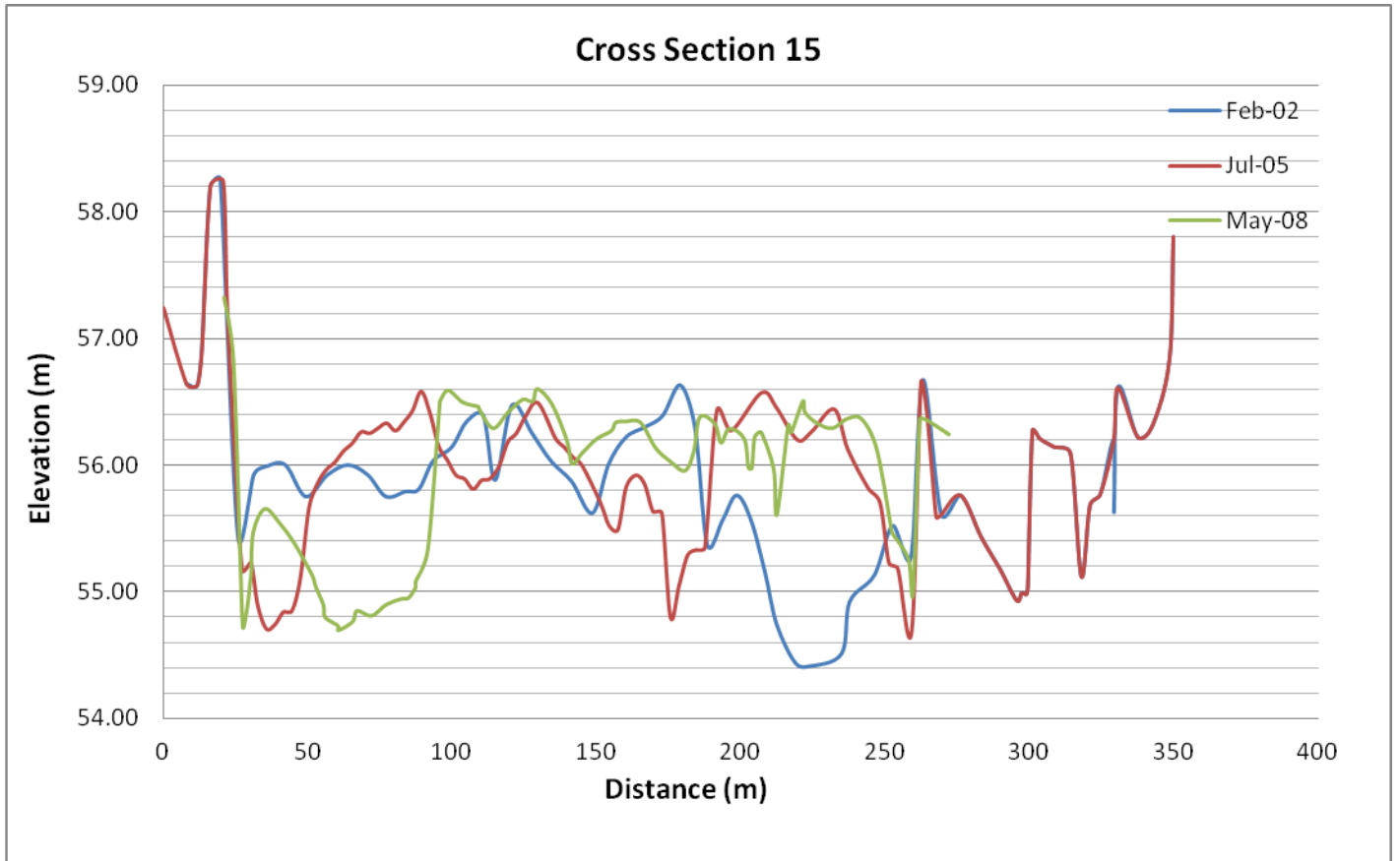


Figure 15. Bed levels at cross-section 6 on February 2002, July 2005, and May 2008.

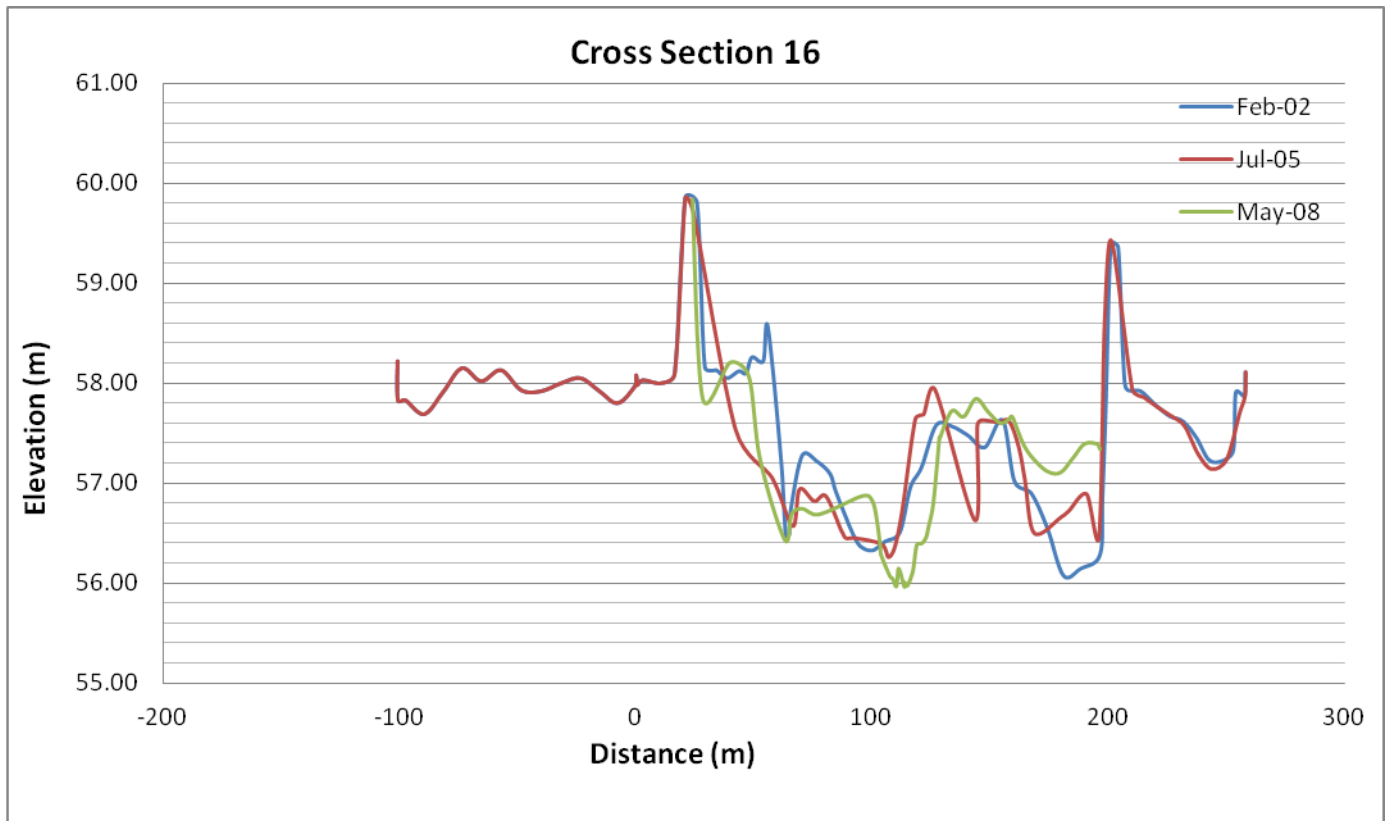


Figure 16. Bed levels at cross-section 16 on February 2002, July 2005, and May 2008.

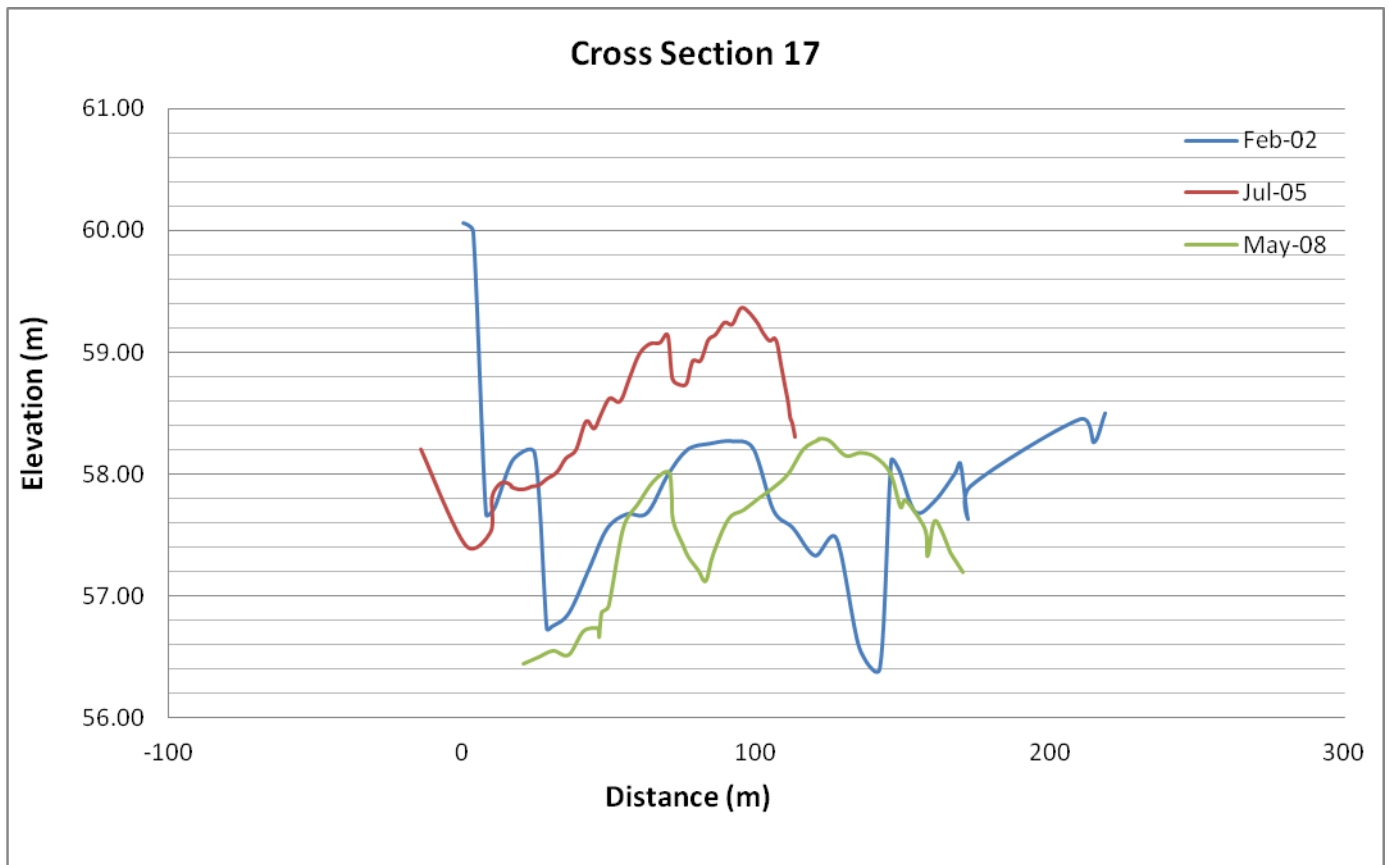


Figure 17. Bed levels at cross-section 17 on February 2002, July 2005, and May 2008.

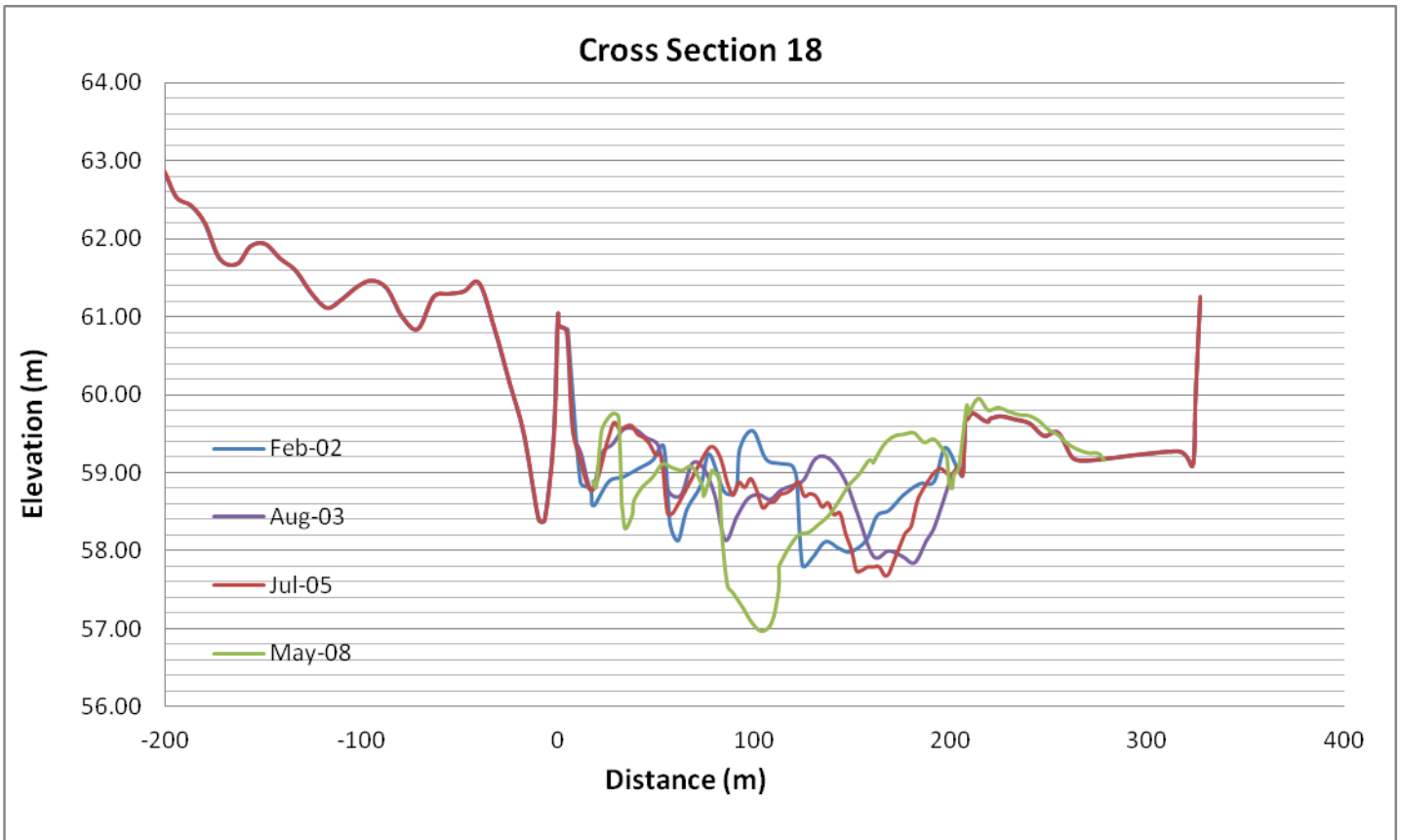


Figure 18. Bed levels at cross-section 18 on February 2002, August 2003, July 2005, and May 2008.

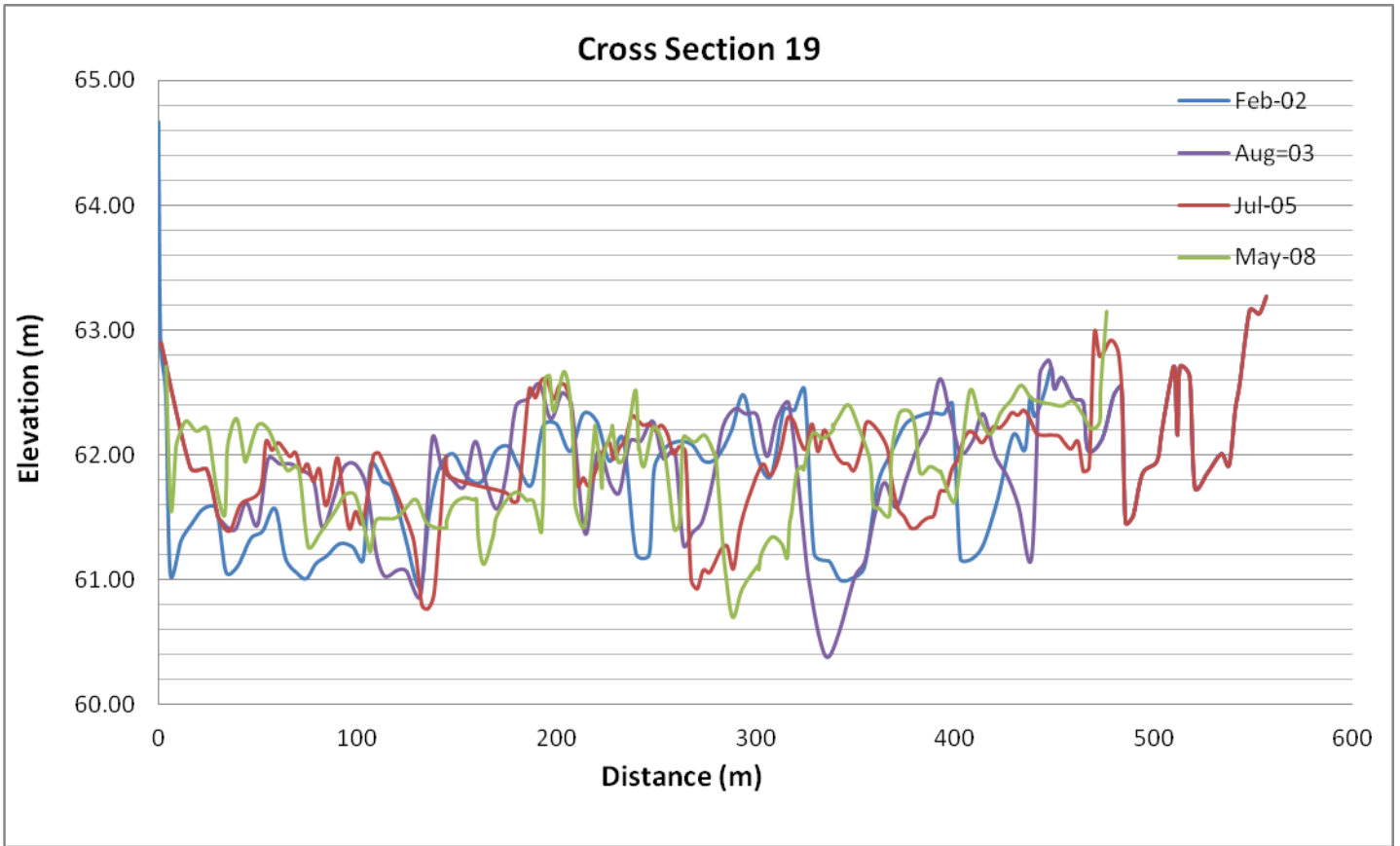


Figure 19. Bed levels at cross-section 19 on February 2002, August 2003, July 2005, and May 2008.

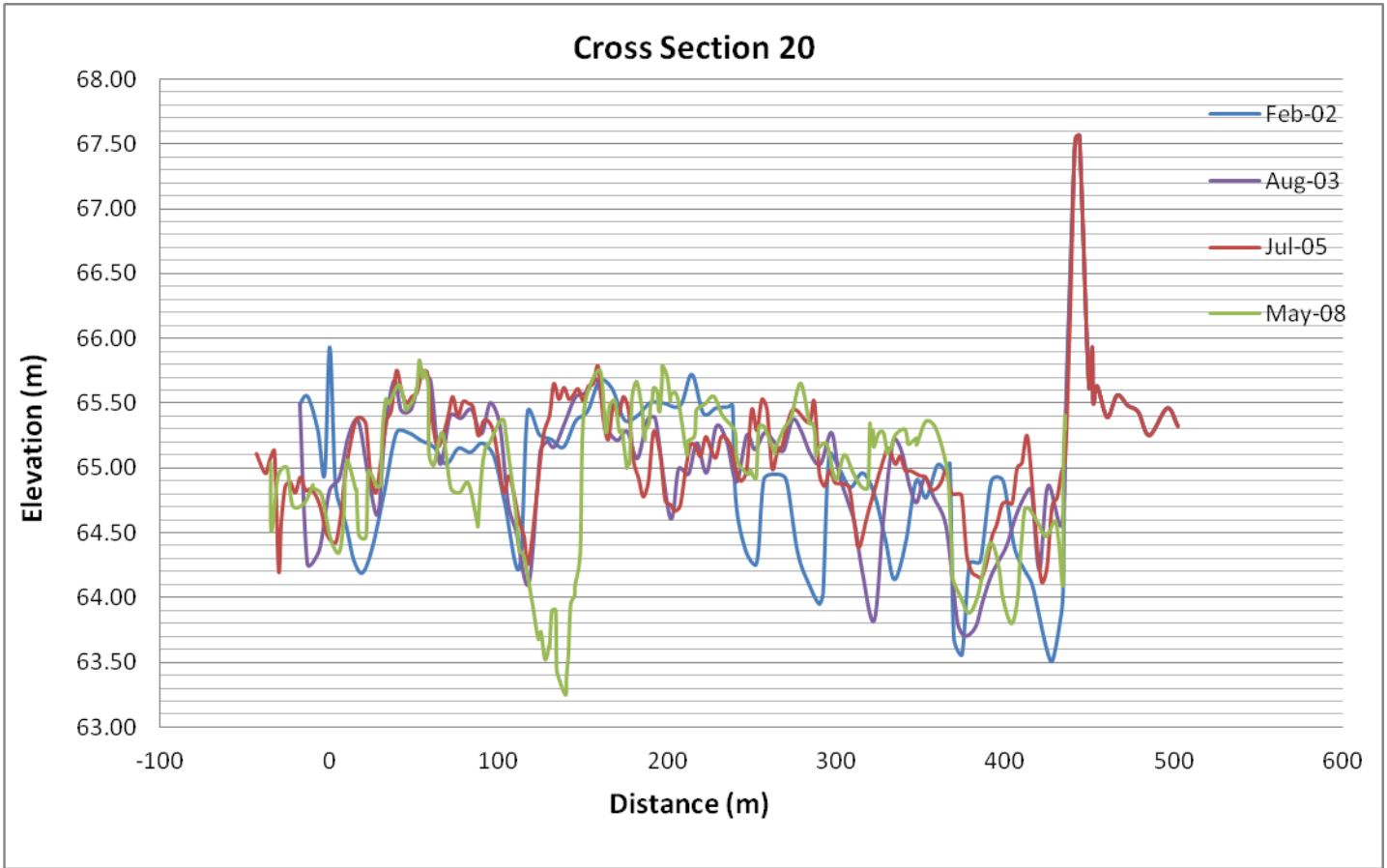


Figure 20. Bed levels at cross-section 20 on February 2002, August 2003, July 2005, and May 2008.

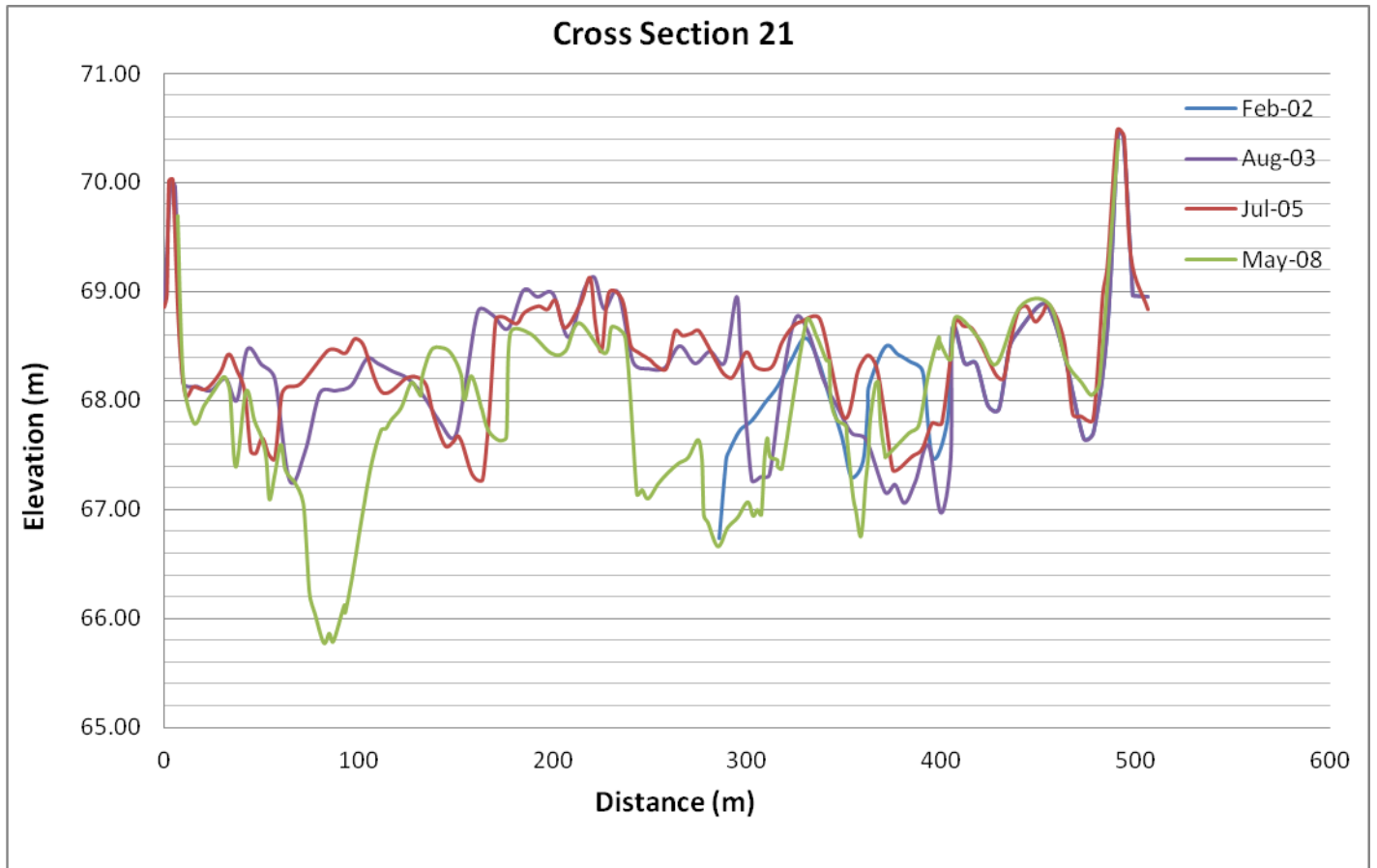


Figure 21. Bed levels at cross-section 21 on February 2002, August 2003, July 2005, and May 2008.

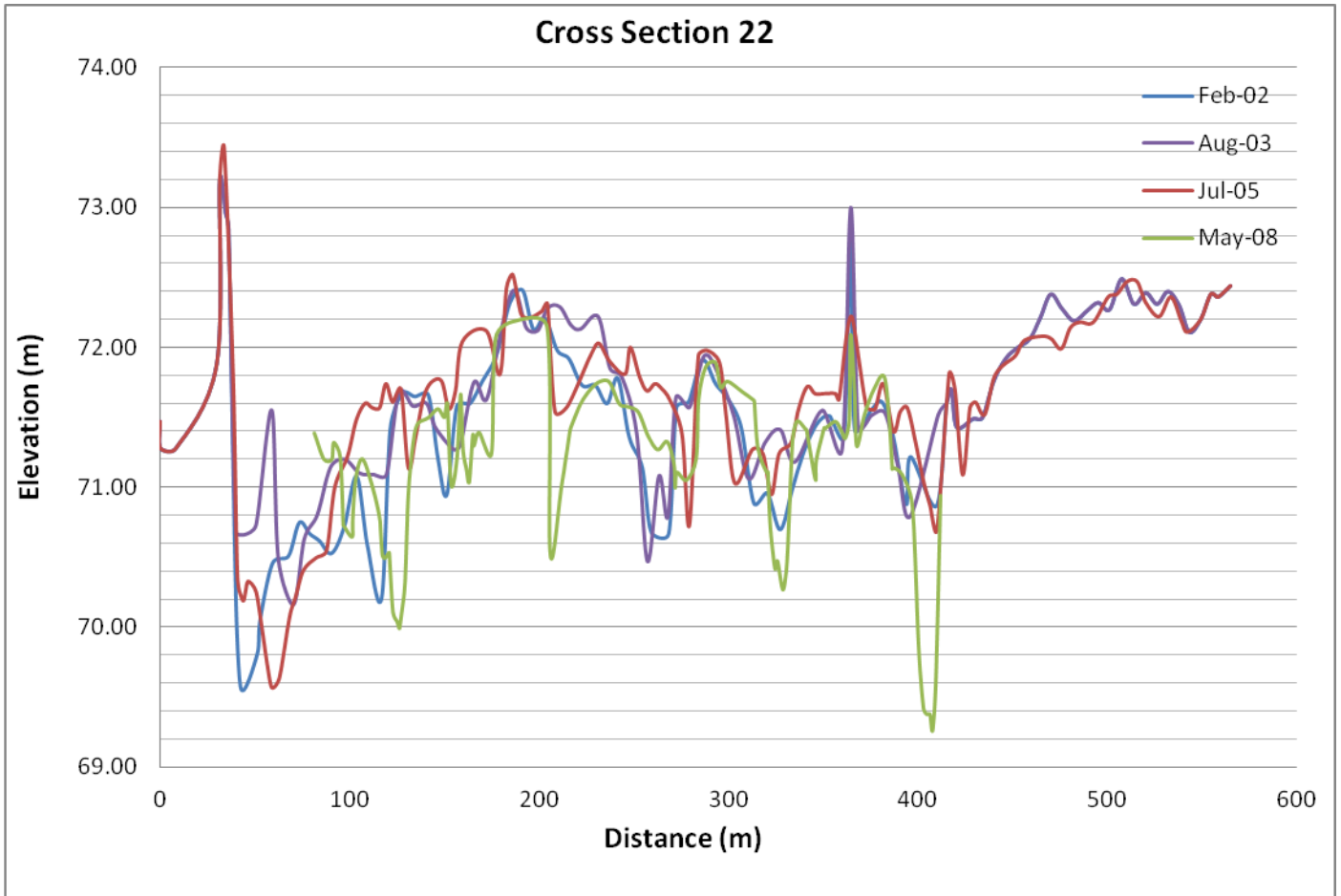


Figure 22. Bed levels at cross-section 22 on February 2002, August 2003, July 2005, and May 2008.

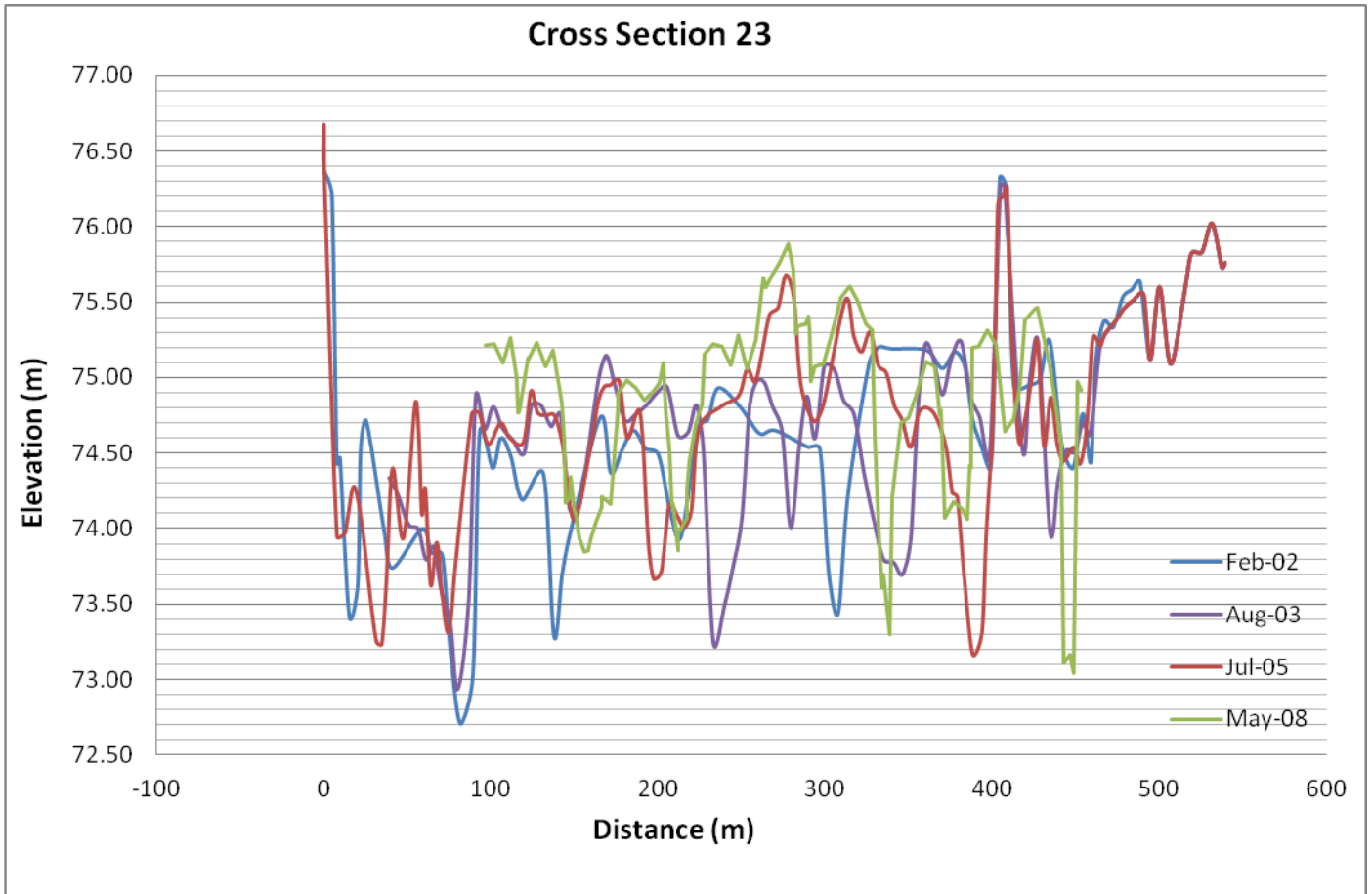


Figure 23. Bed levels at cross-section 23 on February 2002, August 2003, July 2005, and May 2008.

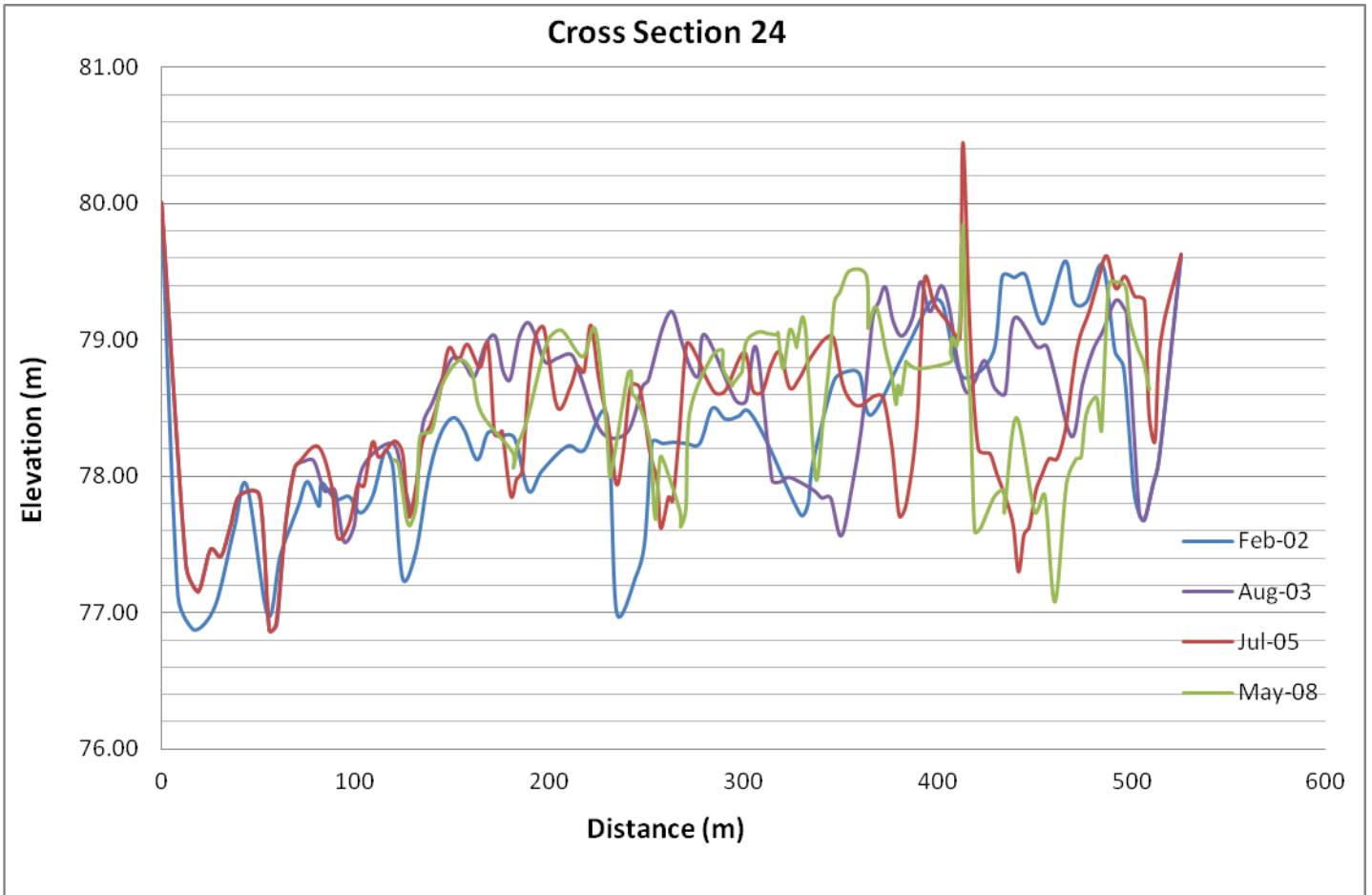


Figure 24. Bed levels at cross-section 24 on February 2002, August 2003, July 2005, and May 2008.

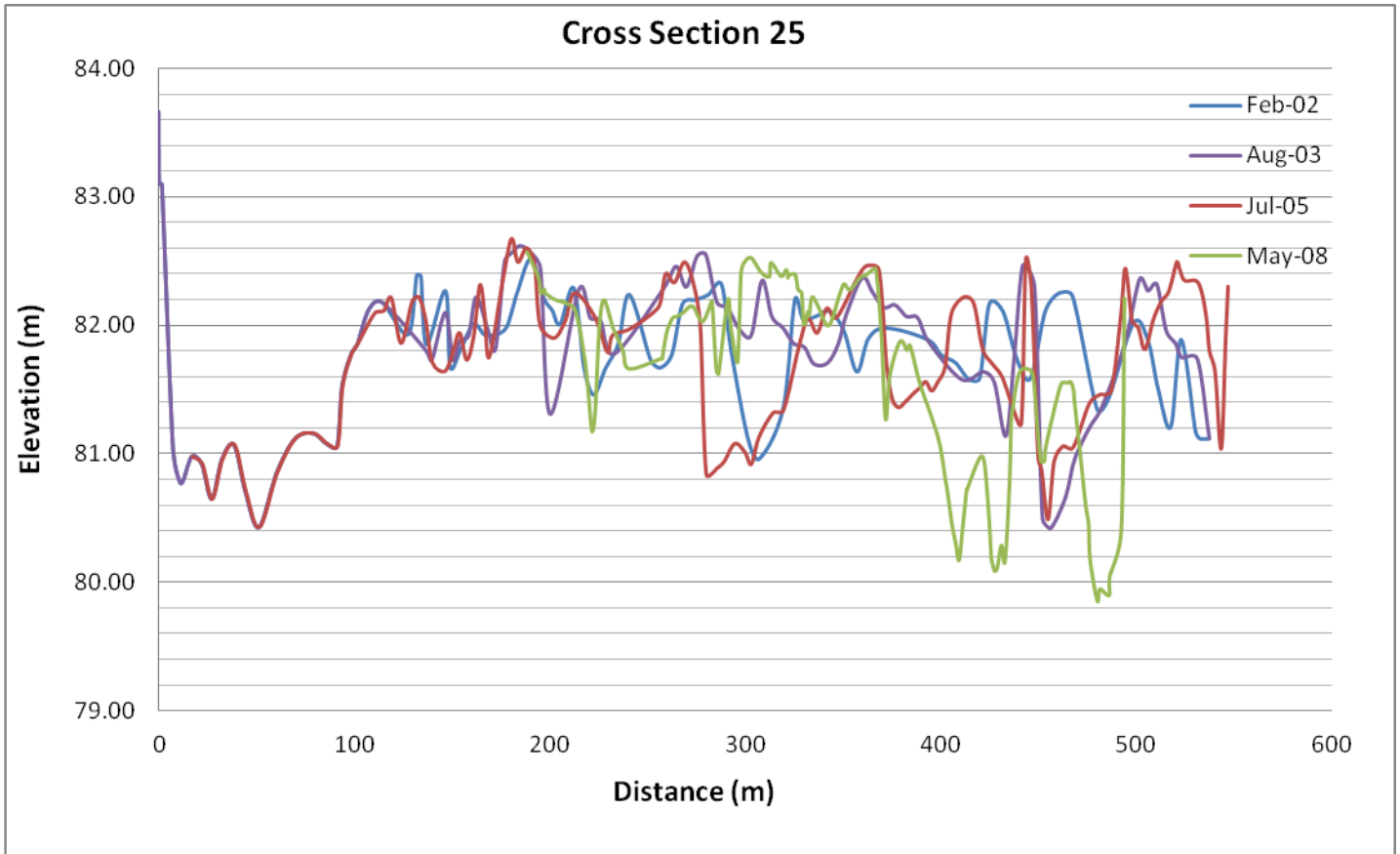


Figure 25. Bed levels at cross-section 25 on February 2002, August 2003, July 2005, and May 2008.

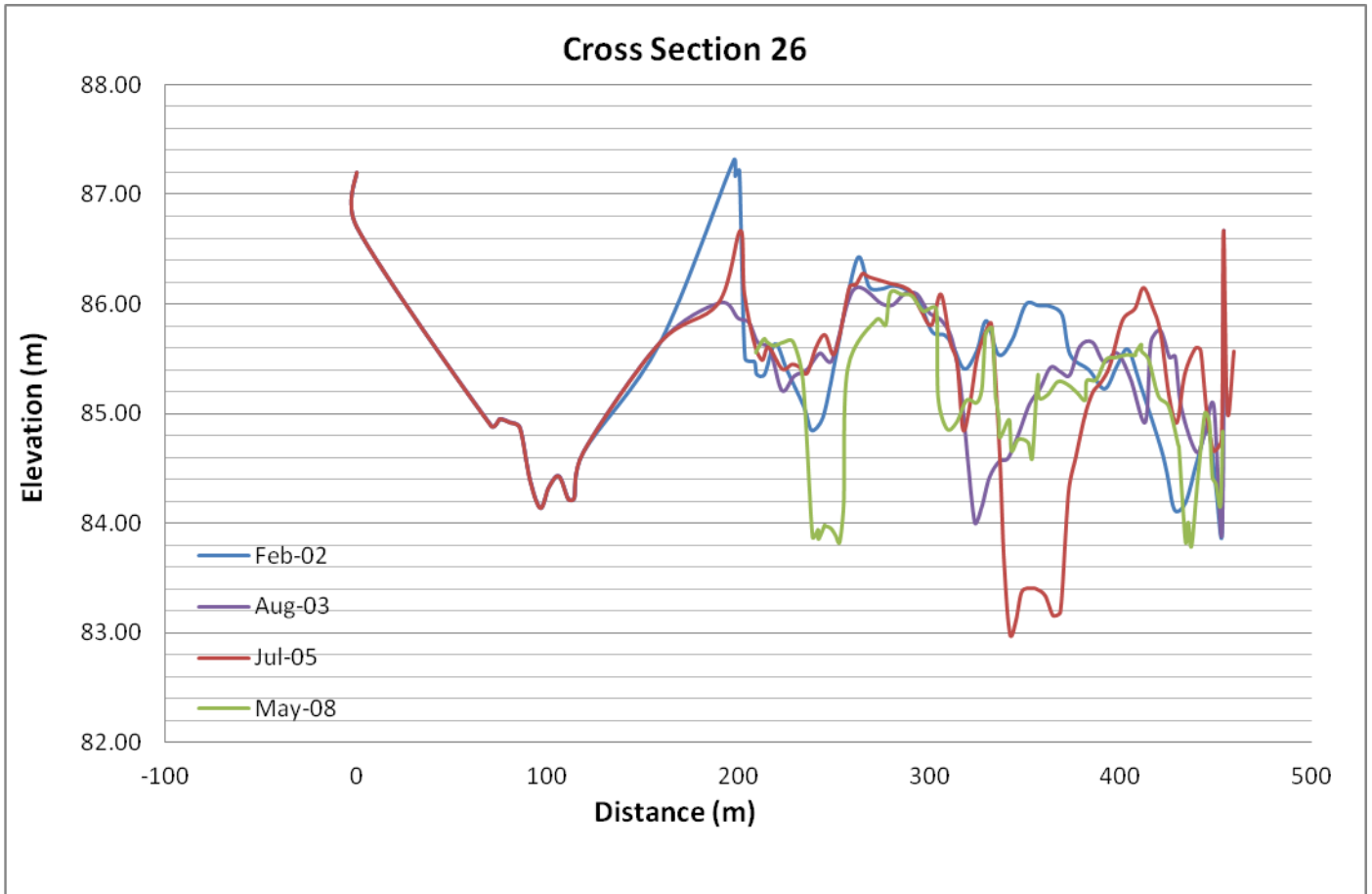


Figure 26. Bed levels at cross-section 26 on February 2002, August 2003, July 2005, and May 2008.

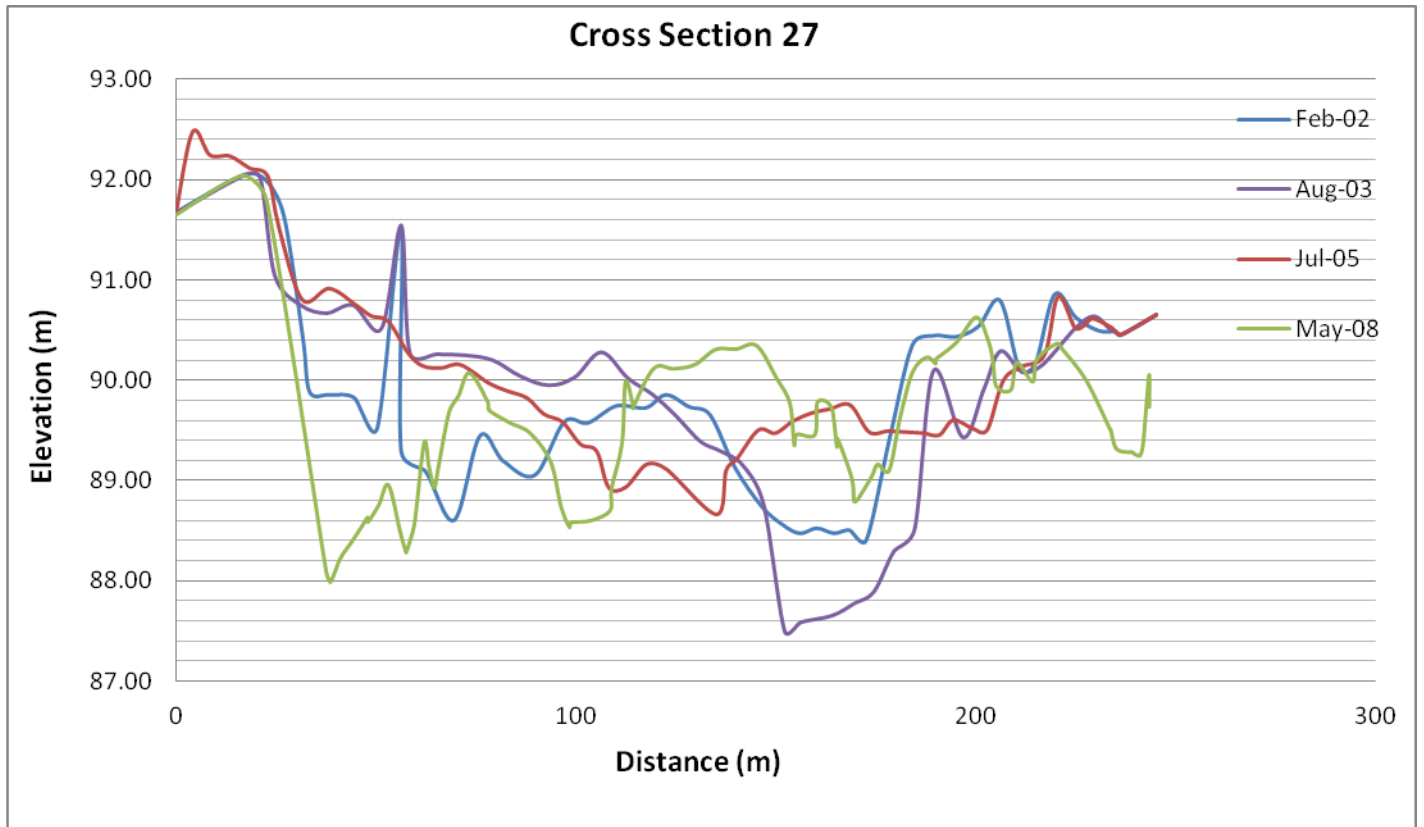


Figure 27. Bed levels at cross-section 27 on February 2002, August 2003, July 2005, and May 2008.

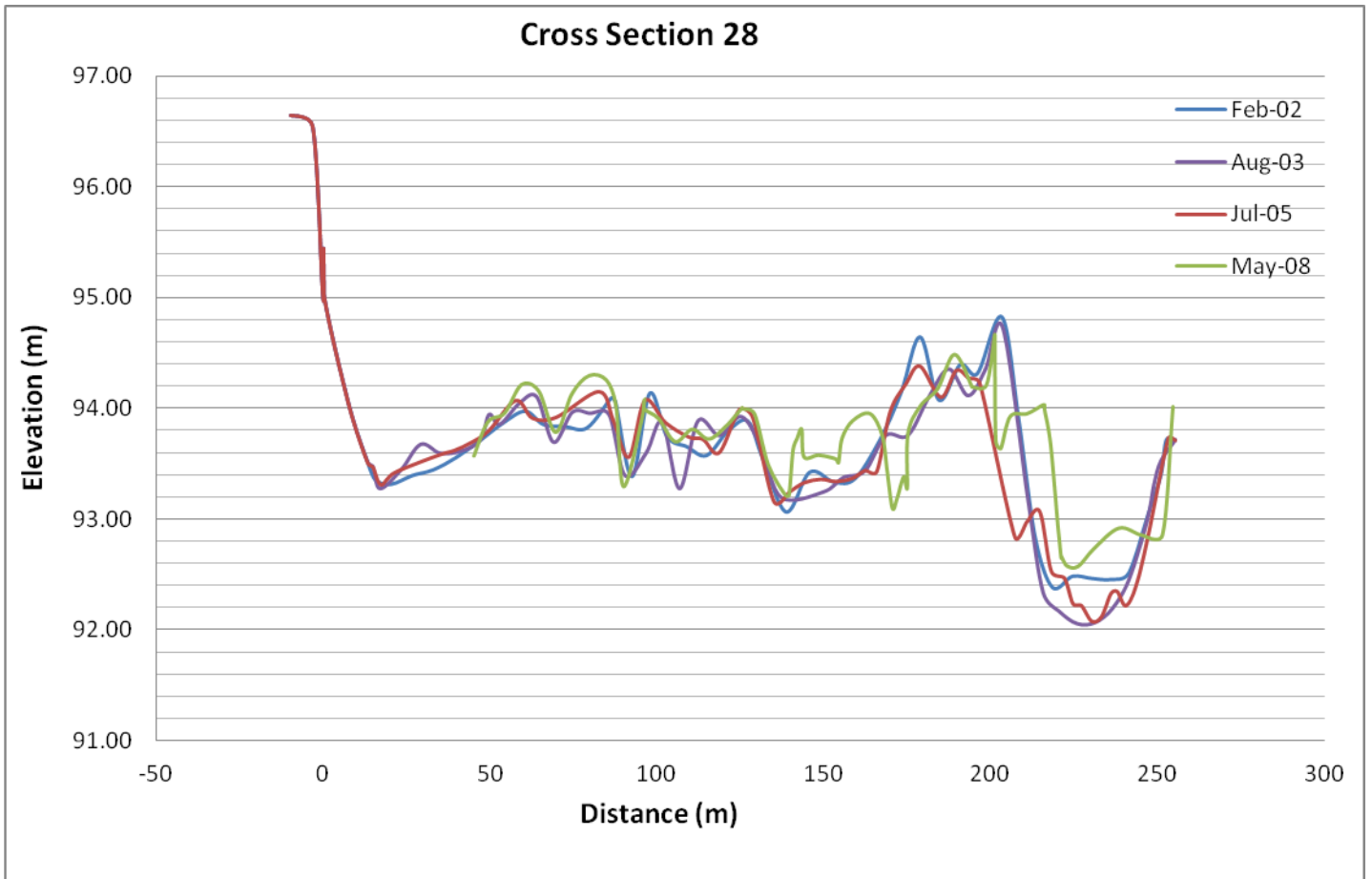


Figure 28. Bed levels at cross-section 28 on February 2002, August 2003, July 2005, and May 2008.

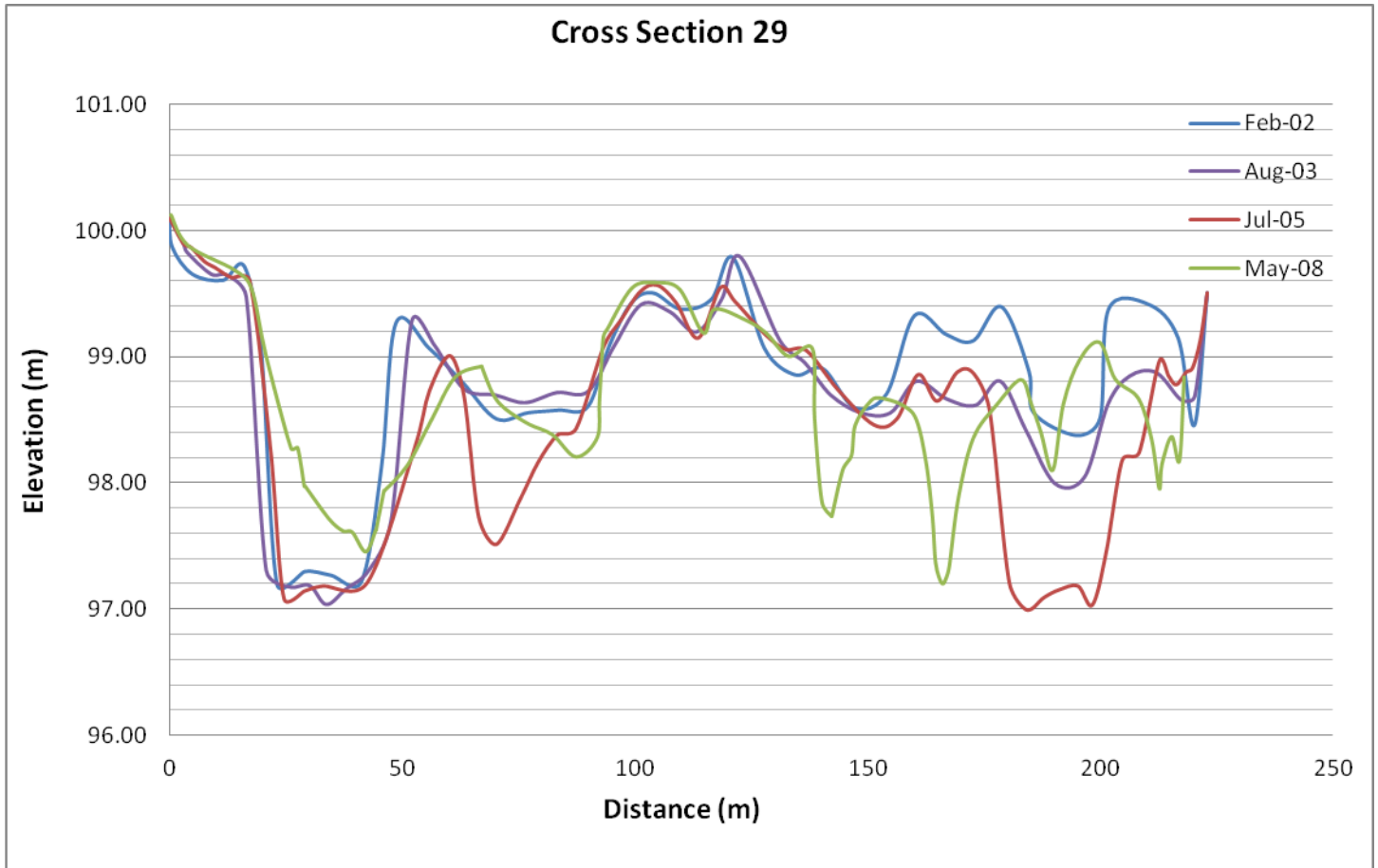


Figure 29. Bed levels at cross-section 29 on February 2002, August 2003, July 2005, and May 2008.

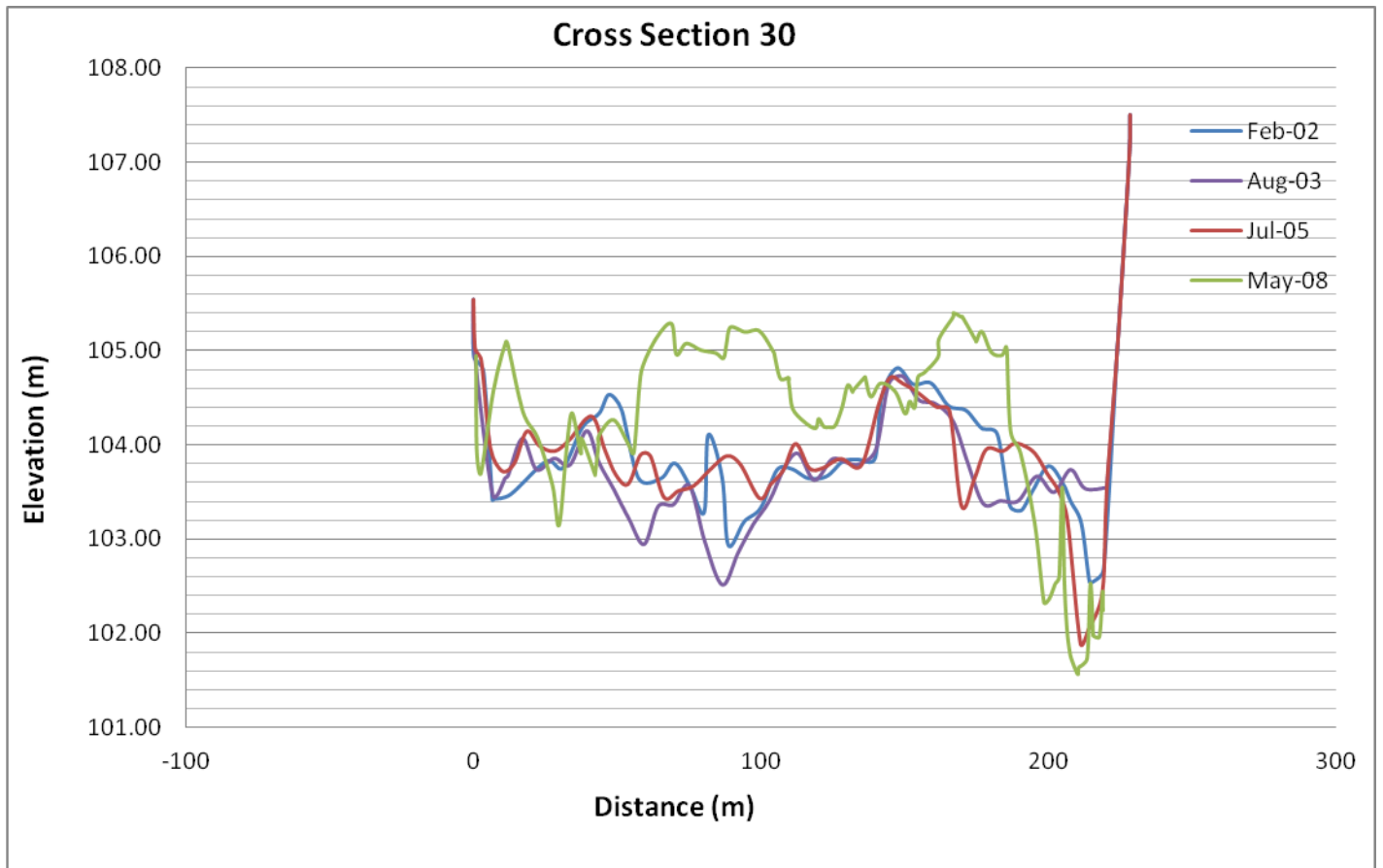


Figure 30. Bed levels at cross-section 30 on February 2002, August 2003, July 2005, and May 2008.

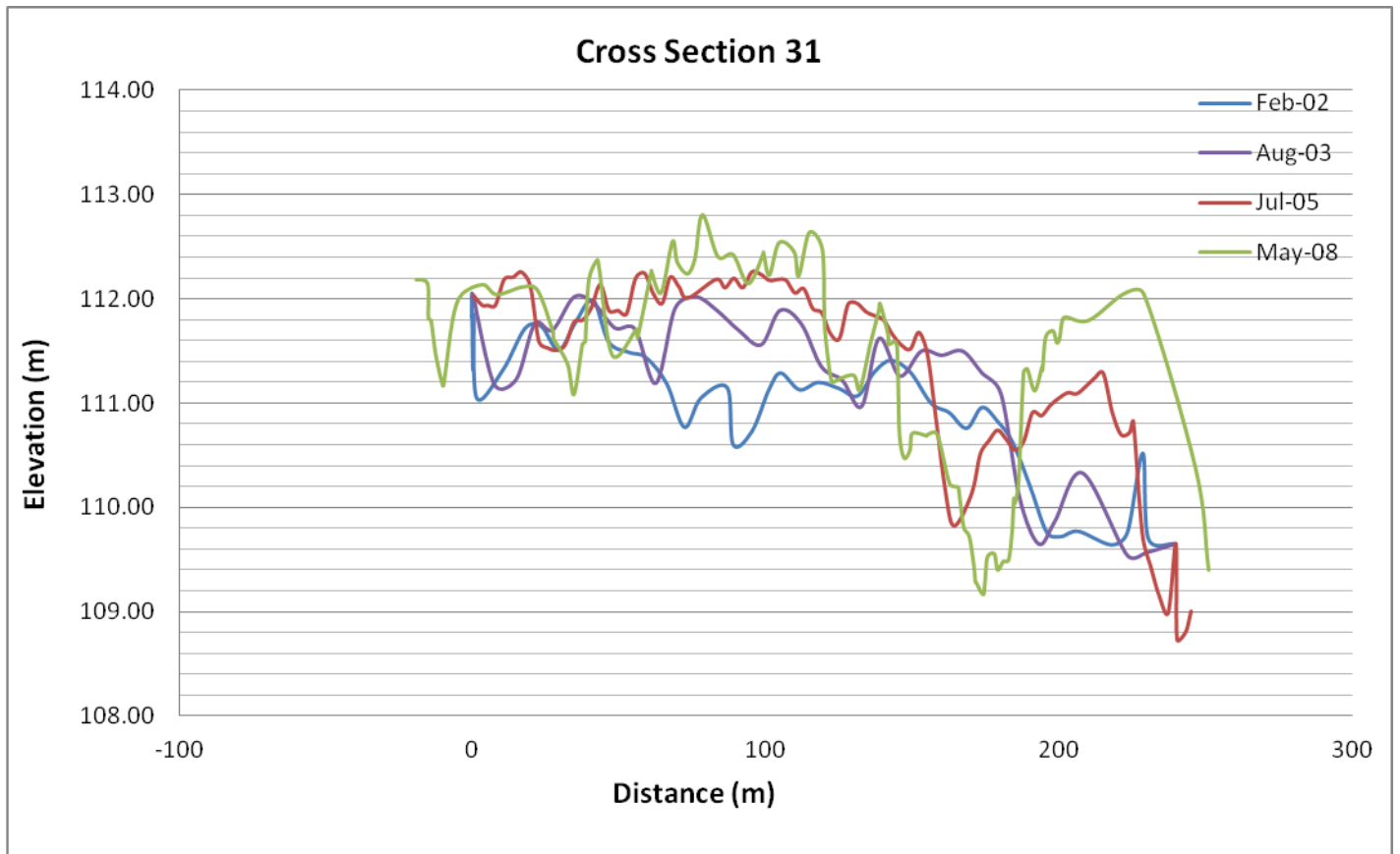


Figure 31. Bed levels at cross-section 31 on February 2002, August 2003, July 2005, and May 2008.

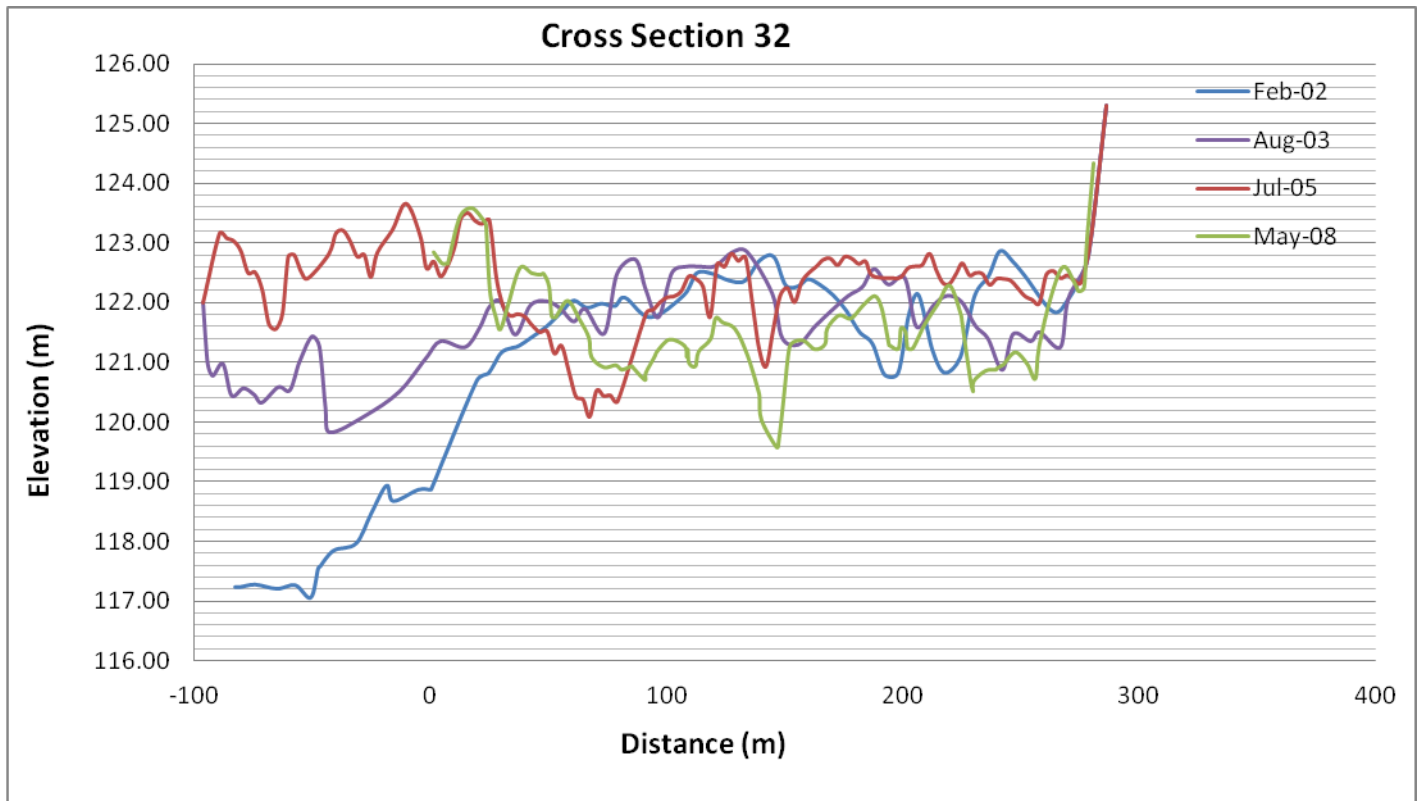


Figure 32. Bed levels at cross-section 32 on February 2002, August 2003, July 2005, and May 2008.

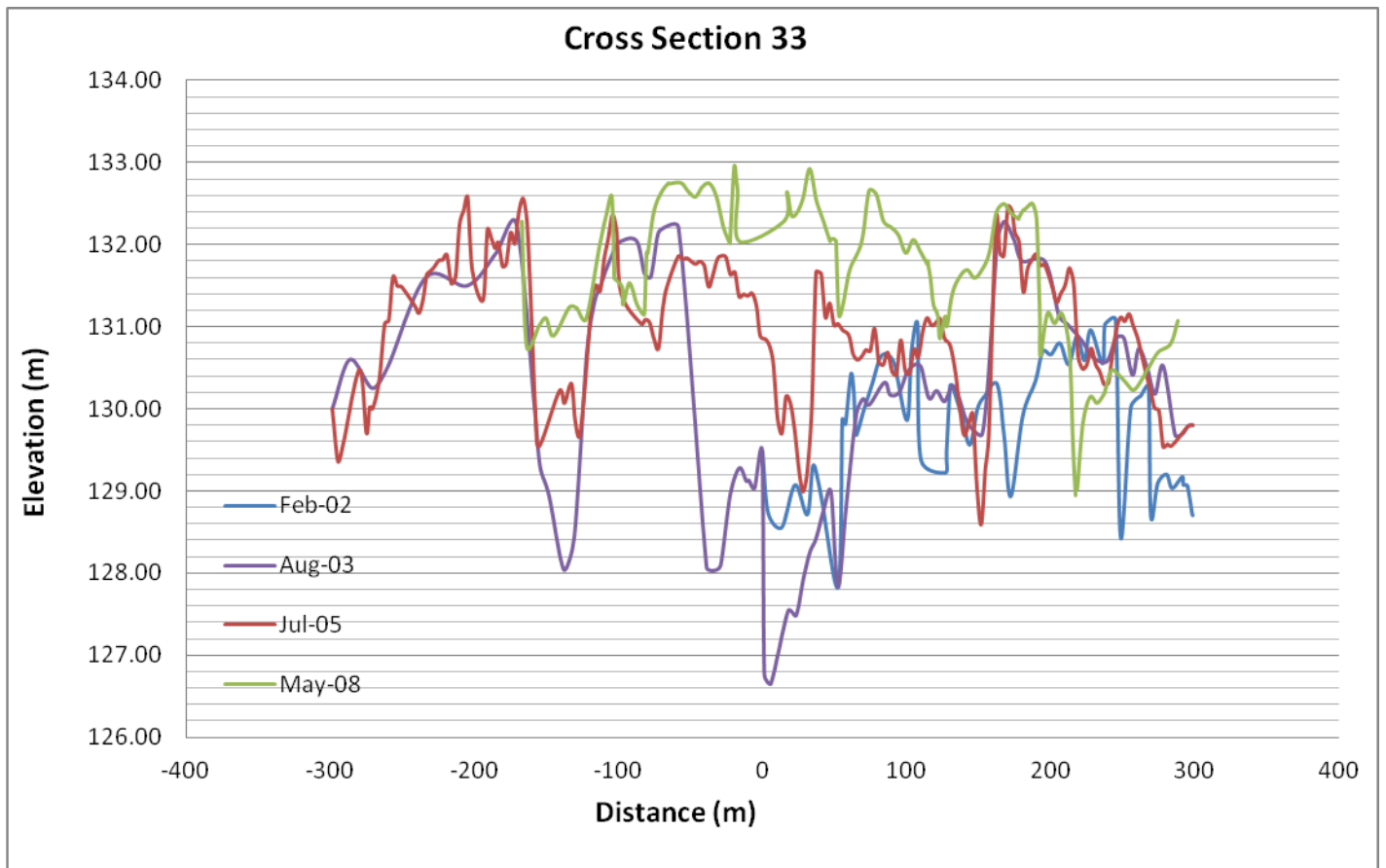


Figure 33. Bed levels at cross-section 33 on February 2002, August 2003, July 2005, and May 2008.

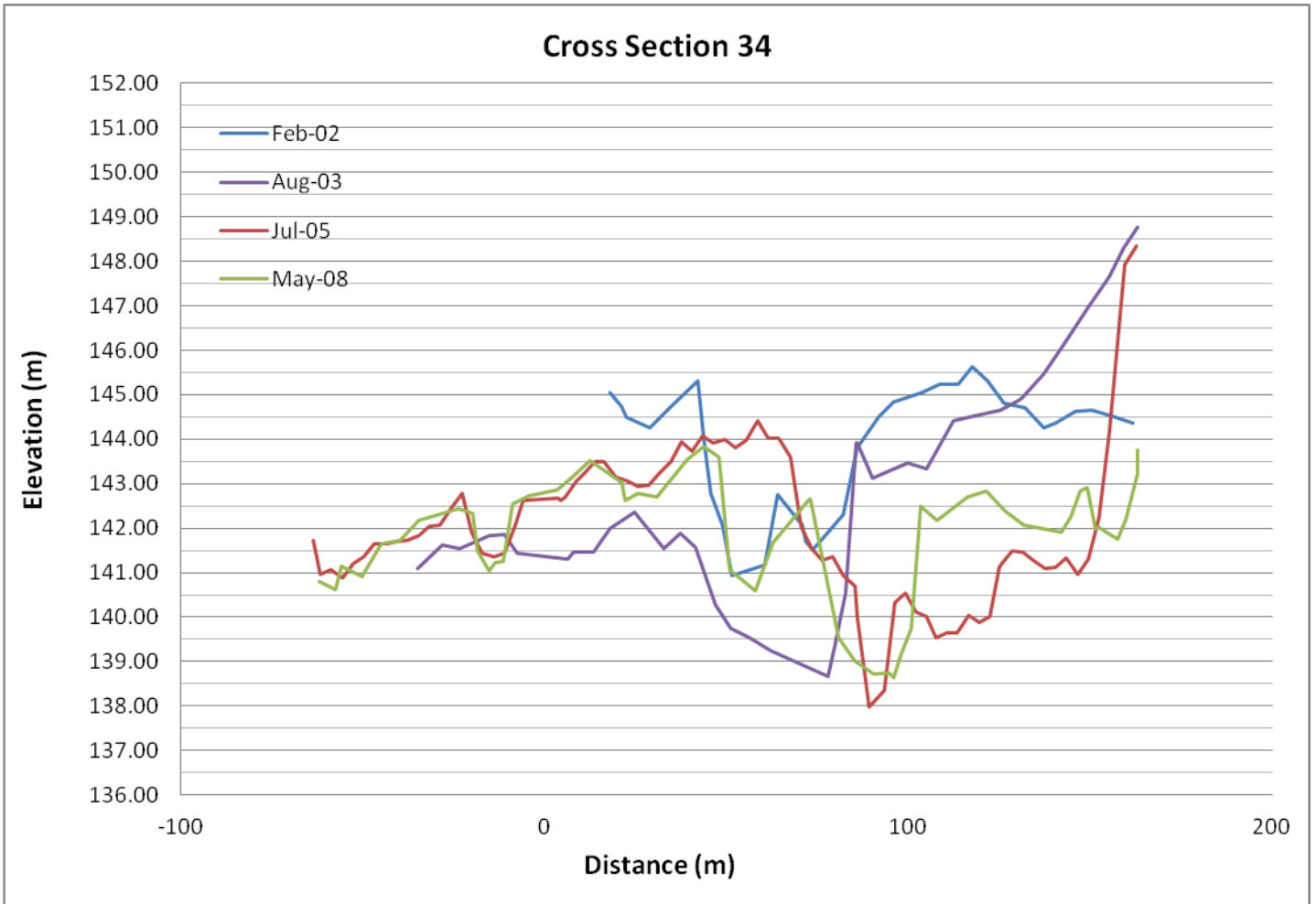


Figure 34. Bed levels at cross-section 34 on February 2002, August 2003, July 2005, and May 2008.

Appendix C: Survey data for the 34 cross-sections.

| <u>ID</u> | <u>Northing</u> | <u>Easting</u> | <u>Elevation</u> | <u>Code</u> |
|-----------|-----------------|----------------|------------------|-------------|
| 1194 | 5223097 | 1393858 | 41.47 | XSEC_1 |
| 1195 | 5223096 | 1393857 | 41.457 | XSEC_1 |
| 1196 | 5223096 | 1393856 | 41.72 | XSEC_1 |
| 1197 | 5223095 | 1393856 | 42.981 | XSEC_1 |
| 1198 | 5223099 | 1393863 | 41.767 | XSEC_1 |
| 1199 | 5223102 | 1393869 | 41.707 | XSEC_1 |
| 1200 | 5223105 | 1393875 | 41.658 | XSEC_1 |
| 1204 | 5223106 | 1393878 | 41.6 | XSEC_1 |
| 1205 | 5223108 | 1393880 | 42.067 | XSEC_1 |
| 1206 | 5223109 | 1393883 | 42.278 | XSEC_1 |
| 1207 | 5223111 | 1393887 | 42.424 | XSEC_1 |
| 1208 | 5223149 | 1393964 | 42.82 | XSEC_1 |
| 1209 | 5223150 | 1393966 | 42.96 | XSEC_1 |
| 1211 | 5223218 | 1394101 | 42.902 | XSEC_1 |
| 1212 | 5223220 | 1394105 | 42.827 | XSEC_1 |
| 1213 | 5223254 | 1394173 | 42.724 | XSEC_1 |
| 1214 | 5223255 | 1394176 | 42.775 | XSEC_1 |
| 1215 | 5223311 | 1394287 | 42.417 | XSEC_1 |
| 1216 | 5223313 | 1394291 | 42.661 | XSEC_1 |
| 1217 | 5223315 | 1394294 | 42.672 | XSEC_1 |
| 13266 | 5223112 | 1393889 | 42.54 | XSEC_1 |
| 13267 | 5223114 | 1393894 | 42.724 | XSEC_1 |
| 13268 | 5223117 | 1393898 | 42.81 | XSEC_1 |
| 13269 | 5223119 | 1393903 | 42.832 | XSEC_1 |
| 13270 | 5223121 | 1393908 | 42.842 | XSEC_1 |
| 13271 | 5223124 | 1393912 | 42.869 | XSEC_1 |
| 13272 | 5223126 | 1393916 | 42.792 | XSEC_1 |
| 13273 | 5223129 | 1393921 | 42.763 | XSEC_1 |
| 13274 | 5223131 | 1393925 | 42.643 | XSEC_1 |
| 13275 | 5223133 | 1393928 | 42.367 | XSEC_1 |
| 13276 | 5223134 | 1393930 | 42.021 | XSEC_1 |
| 13277 | 5223134 | 1393930 | 42.004 | XSEC_1 |
| 13278 | 5223134 | 1393930 | 41.948 | XSEC_1 |
| 13279 | 5223136 | 1393933 | 41.974 | XSEC_1 |
| 13280 | 5223136 | 1393934 | 41.964 | XSEC_1 |
| 13281 | 5223136 | 1393934 | 42.064 | XSEC_1 |
| 13282 | 5223139 | 1393939 | 42.265 | XSEC_1 |
| 13283 | 5223141 | 1393943 | 42.266 | XSEC_1 |
| 13284 | 5223143 | 1393948 | 42.38 | XSEC_1 |
| 13285 | 5223146 | 1393952 | 42.432 | XSEC_1 |

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|-------|---------|---------|--------|--------|
| 13286 | 5223151 | 1393967 | 42.908 | XSEC_1 |
| 13287 | 5223153 | 1393971 | 43.019 | XSEC_1 |
| 13288 | 5223155 | 1393974 | 43.09 | XSEC_1 |
| 13289 | 5223155 | 1393976 | 43.18 | XSEC_1 |
| 13290 | 5223158 | 1393980 | 43.225 | XSEC_1 |
| 13291 | 5223160 | 1393984 | 43.106 | XSEC_1 |
| 13292 | 5223160 | 1393985 | 42.596 | XSEC_1 |
| 13293 | 5223160 | 1393985 | 42.664 | XSEC_1 |
| 13294 | 5223162 | 1393990 | 42.709 | XSEC_1 |
| 13295 | 5223165 | 1393994 | 42.998 | XSEC_1 |
| 13296 | 5223167 | 1393997 | 43.11 | XSEC_1 |
| 13297 | 5223168 | 1393999 | 42.935 | XSEC_1 |
| 13298 | 5223169 | 1394001 | 42.782 | XSEC_1 |
| 13299 | 5223170 | 1394003 | 42.821 | XSEC_1 |
| 13300 | 5223172 | 1394008 | 43.094 | XSEC_1 |
| 13301 | 5223174 | 1394012 | 43.059 | XSEC_1 |
| 13302 | 5223177 | 1394017 | 43.116 | XSEC_1 |
| 13303 | 5223179 | 1394021 | 43.245 | XSEC_1 |
| 13304 | 5223182 | 1394026 | 43.195 | XSEC_1 |
| 13305 | 5223188 | 1394037 | 42.908 | XSEC_1 |
| 13306 | 5223188 | 1394036 | 43.011 | XSEC_1 |
| 13307 | 5223190 | 1394041 | 42.872 | XSEC_1 |
| 13308 | 5223192 | 1394046 | 42.782 | XSEC_1 |
| 13309 | 5223195 | 1394050 | 42.775 | XSEC_1 |
| 13310 | 5223198 | 1394056 | 42.79 | XSEC_1 |
| 13311 | 5223200 | 1394060 | 42.31 | XSEC_1 |
| 13312 | 5223202 | 1394064 | 42.687 | XSEC_1 |
| 13313 | 5223203 | 1394065 | 42.792 | XSEC_1 |
| 13314 | 5223205 | 1394070 | 42.921 | XSEC_1 |
| 13315 | 5223208 | 1394074 | 43.028 | XSEC_1 |
| 13316 | 5223210 | 1394079 | 43.129 | XSEC_1 |
| 13317 | 5223212 | 1394083 | 43.212 | XSEC_1 |
| 13318 | 5223215 | 1394088 | 43.106 | XSEC_1 |
| 13319 | 5223217 | 1394093 | 43.124 | XSEC_1 |
| 13320 | 5223220 | 1394105 | 42.858 | XSEC_1 |
| 13321 | 5223221 | 1394107 | 42.757 | XSEC_1 |
| 13322 | 5223223 | 1394112 | 42.845 | XSEC_1 |
| 13323 | 5223226 | 1394116 | 42.701 | XSEC_1 |
| 13324 | 5223228 | 1394121 | 42.478 | XSEC_1 |
| 13325 | 5223230 | 1394125 | 42.443 | XSEC_1 |
| 13326 | 5223233 | 1394130 | 42.764 | XSEC_1 |
| 13327 | 5223237 | 1394139 | 42.878 | XSEC_1 |
| 13328 | 5223240 | 1394143 | 42.855 | XSEC_1 |

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|-------|---------|---------|--------|--------|
| 13329 | 5223242 | 1394148 | 42.988 | XSEC_1 |
| 13330 | 5223245 | 1394154 | 42.721 | XSEC_1 |
| 13331 | 5223247 | 1394158 | 43.022 | XSEC_1 |
| 13332 | 5223249 | 1394163 | 42.895 | XSEC_1 |
| 13333 | 5223251 | 1394167 | 42.646 | XSEC_1 |
| 13334 | 5223253 | 1394172 | 42.762 | XSEC_1 |
| 13335 | 5223257 | 1394178 | 42.9 | XSEC_1 |
| 13336 | 5223259 | 1394183 | 42.943 | XSEC_1 |
| 13337 | 5223261 | 1394188 | 43.051 | XSEC_1 |
| 13338 | 5223265 | 1394194 | 42.96 | XSEC_1 |
| 13339 | 5223268 | 1394199 | 43.073 | XSEC_1 |
| 13340 | 5223272 | 1394206 | 42.816 | XSEC_1 |
| 13341 | 5223273 | 1394208 | 42.531 | XSEC_1 |
| 13342 | 5223275 | 1394211 | 42.531 | XSEC_1 |
| 13343 | 5223277 | 1394215 | 42.593 | XSEC_1 |
| 13344 | 5223281 | 1394222 | 42.476 | XSEC_1 |
| 13345 | 5223282 | 1394223 | 42.301 | XSEC_1 |
| 13346 | 5223283 | 1394227 | 42.255 | XSEC_1 |
| 13347 | 5223286 | 1394231 | 42.087 | XSEC_1 |
| 13348 | 5223288 | 1394235 | 42.04 | XSEC_1 |
| 13349 | 5223288 | 1394236 | 42.481 | XSEC_1 |
| 13350 | 5223289 | 1394237 | 42.741 | XSEC_1 |
| 13351 | 5223291 | 1394240 | 42.783 | XSEC_1 |
| 13352 | 5223294 | 1394248 | 42.744 | XSEC_1 |
| 13353 | 5223296 | 1394252 | 42.889 | XSEC_1 |
| 13354 | 5223298 | 1394257 | 42.814 | XSEC_1 |
| 13355 | 5223300 | 1394262 | 42.61 | XSEC_1 |
| 13356 | 5223303 | 1394266 | 42.351 | XSEC_1 |
| 13357 | 5223305 | 1394271 | 42.31 | XSEC_1 |
| 13358 | 5223307 | 1394276 | 42.191 | XSEC_1 |
| 13359 | 5223316 | 1394296 | 42.77 | XSEC_1 |
| 13360 | 5223318 | 1394301 | 42.786 | XSEC_1 |
| 13361 | 5223321 | 1394305 | 42.835 | XSEC_1 |
| 13362 | 5223323 | 1394310 | 42.82 | XSEC_1 |
| 13363 | 5223325 | 1394315 | 42.798 | XSEC_1 |
| 13364 | 5223329 | 1394322 | 42.725 | XSEC_1 |
| 13365 | 5223331 | 1394326 | 42.762 | XSEC_1 |
| 13366 | 5223333 | 1394331 | 42.58 | XSEC_1 |
| 13367 | 5223334 | 1394333 | 42.055 | XSEC_1 |
| 13368 | 5223335 | 1394335 | 42.058 | XSEC_1 |
| 13369 | 5223338 | 1394340 | 42.053 | XSEC_1 |
| 13370 | 5223342 | 1394346 | 41.761 | XSEC_1 |
| 13371 | 5223343 | 1394349 | 41.298 | XSEC_1 |

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|-------|---------|---------|--------|--------|
| 13372 | 5223345 | 1394352 | 41.269 | XSEC_1 |
| 13373 | 5223345 | 1394352 | 41.351 | XSEC_1 |
| 13374 | 5223346 | 1394353 | 41.419 | XSEC_1 |
| 1179 | 5222805 | 1394178 | 44.151 | XSEC_2 |
| 1180 | 5222806 | 1394177 | 43.611 | XSEC_2 |
| 1181 | 5222806 | 1394177 | 43.422 | XSEC_2 |
| 1182 | 5222803 | 1394172 | 43.88 | XSEC_2 |
| 1183 | 5222801 | 1394169 | 44.004 | XSEC_2 |
| 1184 | 5222762 | 1394091 | 44.229 | XSEC_2 |
| 1185 | 5222760 | 1394086 | 44.058 | XSEC_2 |
| 1186 | 5222758 | 1394082 | 44.395 | XSEC_2 |
| 1187 | 5222754 | 1394074 | 44.461 | XSEC_2 |
| 1188 | 5222721 | 1394008 | 43.419 | XSEC_2 |
| 1189 | 5222717 | 1394002 | 43.41 | XSEC_2 |
| 1190 | 5222692 | 1393951 | 43.893 | XSEC_2 |
| 1191 | 5222692 | 1393951 | 44.479 | XSEC_2 |
| 1192 | 5222691 | 1393949 | 45.006 | XSEC_2 |
| 1193 | 5222690 | 1393947 | 45.115 | XSEC_2 |
| 13218 | 5222800 | 1394166 | 44.112 | XSEC_2 |
| 13219 | 5222798 | 1394161 | 44.148 | XSEC_2 |
| 13220 | 5222796 | 1394157 | 43.921 | XSEC_2 |
| 13221 | 5222794 | 1394152 | 43.946 | XSEC_2 |
| 13222 | 5222792 | 1394147 | 43.941 | XSEC_2 |
| 13223 | 5222789 | 1394143 | 44.227 | XSEC_2 |
| 13224 | 5222787 | 1394138 | 44.281 | XSEC_2 |
| 13225 | 5222785 | 1394133 | 44.4 | XSEC_2 |
| 13226 | 5222782 | 1394129 | 44.539 | XSEC_2 |
| 13227 | 5222780 | 1394124 | 44.531 | XSEC_2 |
| 13228 | 5222778 | 1394120 | 44.5 | XSEC_2 |
| 13229 | 5222775 | 1394115 | 44.313 | XSEC_2 |
| 13230 | 5222774 | 1394112 | 44.167 | XSEC_2 |
| 13231 | 5222773 | 1394111 | 44.295 | XSEC_2 |
| 13232 | 5222770 | 1394106 | 44.653 | XSEC_2 |
| 13233 | 5222770 | 1394106 | 44.655 | XSEC_2 |
| 13234 | 5222768 | 1394102 | 44.785 | XSEC_2 |
| 13235 | 5222766 | 1394097 | 44.71 | XSEC_2 |
| 13236 | 5222764 | 1394093 | 44.678 | XSEC_2 |
| 13237 | 5222754 | 1394074 | 44.466 | XSEC_2 |
| 13238 | 5222752 | 1394070 | 44.579 | XSEC_2 |
| 13239 | 5222749 | 1394065 | 44.64 | XSEC_2 |
| 13240 | 5222747 | 1394060 | 44.774 | XSEC_2 |
| 13241 | 5222745 | 1394056 | 44.577 | XSEC_2 |
| 13242 | 5222743 | 1394051 | 44.268 | XSEC_2 |

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|-------|---------|---------|--------|--------|
| 13243 | 5222740 | 1394046 | 43.977 | XSEC_2 |
| 13244 | 5222738 | 1394042 | 43.859 | XSEC_2 |
| 13245 | 5222736 | 1394037 | 43.657 | XSEC_2 |
| 13246 | 5222733 | 1394032 | 43.463 | XSEC_2 |
| 13247 | 5222731 | 1394028 | 43.329 | XSEC_2 |
| 13248 | 5222730 | 1394026 | 43.178 | XSEC_2 |
| 13249 | 5222729 | 1394023 | 43.184 | XSEC_2 |
| 13250 | 5222727 | 1394021 | 43.231 | XSEC_2 |
| 13251 | 5222727 | 1394019 | 43.347 | XSEC_2 |
| 13252 | 5222725 | 1394014 | 43.399 | XSEC_2 |
| 13253 | 5222715 | 1393997 | 43.3 | XSEC_2 |
| 13254 | 5222713 | 1393992 | 43.267 | XSEC_2 |
| 13255 | 5222711 | 1393989 | 42.962 | XSEC_2 |
| 13256 | 5222710 | 1393988 | 42.941 | XSEC_2 |
| 13257 | 5222710 | 1393988 | 43.129 | XSEC_2 |
| 13258 | 5222708 | 1393984 | 43.869 | XSEC_2 |
| 13259 | 5222708 | 1393983 | 44.047 | XSEC_2 |
| 13260 | 5222705 | 1393979 | 44.109 | XSEC_2 |
| 13261 | 5222703 | 1393974 | 44.126 | XSEC_2 |
| 13262 | 5222701 | 1393970 | 44.167 | XSEC_2 |
| 13263 | 5222698 | 1393965 | 44.307 | XSEC_2 |
| 13264 | 5222696 | 1393960 | 44.413 | XSEC_2 |
| 13265 | 5222693 | 1393956 | 44.367 | XSEC_2 |
| 1158 | 5222303 | 1394073 | 44.597 | XSEC_3 |
| 1159 | 5222303 | 1394074 | 44.187 | XSEC_3 |
| 1160 | 5222304 | 1394075 | 45.901 | XSEC_3 |
| 1161 | 5222305 | 1394077 | 45.742 | XSEC_3 |
| 1162 | 5222307 | 1394079 | 44.179 | XSEC_3 |
| 1163 | 5222307 | 1394081 | 44.359 | XSEC_3 |
| 1164 | 5222308 | 1394085 | 44.34 | XSEC_3 |
| 1165 | 5222309 | 1394086 | 44.107 | XSEC_3 |
| 1166 | 5222310 | 1394087 | 44.145 | XSEC_3 |
| 1167 | 5222310 | 1394088 | 44.229 | XSEC_3 |
| 1168 | 5222311 | 1394090 | 45.057 | XSEC_3 |
| 1169 | 5222313 | 1394093 | 45.41 | XSEC_3 |
| 1170 | 5222312 | 1394091 | 45.118 | XSEC_3 |
| 1171 | 5222359 | 1394185 | 45.746 | XSEC_3 |
| 1172 | 5222361 | 1394188 | 45.733 | XSEC_3 |
| 1173 | 5222387 | 1394240 | 45.637 | XSEC_3 |
| 1174 | 5222388 | 1394242 | 46.2 | XSEC_3 |
| 1175 | 5222388 | 1394243 | 46.162 | XSEC_3 |
| 1176 | 5222448 | 1394361 | 46.123 | XSEC_3 |
| 1177 | 5222450 | 1394366 | 46.236 | XSEC_3 |

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|-------|---------|---------|--------|--------|
| 1178 | 5222497 | 1394460 | 46.483 | XSEC_3 |
| 13121 | 5222315 | 1394096 | 45.554 | XSEC_3 |
| 13122 | 5222316 | 1394099 | 45.587 | XSEC_3 |
| 13123 | 5222317 | 1394101 | 45.568 | XSEC_3 |
| 13124 | 5222319 | 1394106 | 45.748 | XSEC_3 |
| 13125 | 5222321 | 1394110 | 46.04 | XSEC_3 |
| 13126 | 5222324 | 1394115 | 46.198 | XSEC_3 |
| 13127 | 5222326 | 1394120 | 46.191 | XSEC_3 |
| 13128 | 5222328 | 1394124 | 46.098 | XSEC_3 |
| 13129 | 5222330 | 1394129 | 45.972 | XSEC_3 |
| 13130 | 5222332 | 1394131 | 45.833 | XSEC_3 |
| 13131 | 5222332 | 1394132 | 45.642 | XSEC_3 |
| 13132 | 5222333 | 1394133 | 45.624 | XSEC_3 |
| 13133 | 5222334 | 1394136 | 45.775 | XSEC_3 |
| 13134 | 5222335 | 1394138 | 45.812 | XSEC_3 |
| 13135 | 5222338 | 1394142 | 45.931 | XSEC_3 |
| 13136 | 5222341 | 1394147 | 45.943 | XSEC_3 |
| 13137 | 5222343 | 1394152 | 45.991 | XSEC_3 |
| 13138 | 5222346 | 1394156 | 45.932 | XSEC_3 |
| 13139 | 5222348 | 1394161 | 45.895 | XSEC_3 |
| 13140 | 5222350 | 1394165 | 45.888 | XSEC_3 |
| 13141 | 5222353 | 1394170 | 45.794 | XSEC_3 |
| 13142 | 5222355 | 1394174 | 45.817 | XSEC_3 |
| 13143 | 5222357 | 1394179 | 45.777 | XSEC_3 |
| 13144 | 5222360 | 1394183 | 45.816 | XSEC_3 |
| 13145 | 5222361 | 1394189 | 45.745 | XSEC_3 |
| 13146 | 5222364 | 1394194 | 45.725 | XSEC_3 |
| 13147 | 5222366 | 1394198 | 45.487 | XSEC_3 |
| 13148 | 5222368 | 1394203 | 45.295 | XSEC_3 |
| 13149 | 5222370 | 1394206 | 45 | XSEC_3 |
| 13150 | 5222371 | 1394207 | 44.826 | XSEC_3 |
| 13151 | 5222372 | 1394208 | 44.614 | XSEC_3 |
| 13152 | 5222373 | 1394210 | 44.423 | XSEC_3 |
| 13153 | 5222373 | 1394211 | 44.433 | XSEC_3 |
| 13154 | 5222373 | 1394212 | 44.552 | XSEC_3 |
| 13155 | 5222375 | 1394216 | 44.682 | XSEC_3 |
| 13156 | 5222376 | 1394218 | 44.677 | XSEC_3 |
| 13157 | 5222378 | 1394221 | 45.567 | XSEC_3 |
| 13158 | 5222378 | 1394221 | 45.516 | XSEC_3 |
| 13159 | 5222380 | 1394225 | 45.553 | XSEC_3 |
| 13160 | 5222382 | 1394230 | 45.462 | XSEC_3 |
| 13161 | 5222385 | 1394234 | 45.361 | XSEC_3 |
| 13162 | 5222387 | 1394239 | 45.533 | XSEC_3 |

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|-------|---------|---------|--------|--------|
| 13163 | 5222389 | 1394244 | 45.678 | XSEC_3 |
| 13164 | 5222389 | 1394245 | 45.652 | XSEC_3 |
| 13165 | 5222390 | 1394246 | 45.498 | XSEC_3 |
| 13166 | 5222392 | 1394250 | 45.452 | XSEC_3 |
| 13167 | 5222394 | 1394255 | 45.399 | XSEC_3 |
| 13168 | 5222396 | 1394259 | 45.29 | XSEC_3 |
| 13169 | 5222398 | 1394264 | 45.238 | XSEC_3 |
| 13170 | 5222401 | 1394268 | 45.234 | XSEC_3 |
| 13171 | 5222403 | 1394273 | 45.364 | XSEC_3 |
| 13172 | 5222405 | 1394277 | 45.678 | XSEC_3 |
| 13173 | 5222407 | 1394280 | 45.829 | XSEC_3 |
| 13174 | 5222408 | 1394282 | 45.97 | XSEC_3 |
| 13175 | 5222410 | 1394286 | 46.114 | XSEC_3 |
| 13176 | 5222412 | 1394291 | 46.041 | XSEC_3 |
| 13177 | 5222415 | 1394295 | 46.144 | XSEC_3 |
| 13178 | 5222417 | 1394300 | 46.206 | XSEC_3 |
| 13179 | 5222420 | 1394304 | 46.226 | XSEC_3 |
| 13180 | 5222423 | 1394309 | 46.173 | XSEC_3 |
| 13181 | 5222425 | 1394313 | 46.092 | XSEC_3 |
| 13182 | 5222427 | 1394318 | 45.883 | XSEC_3 |
| 13183 | 5222430 | 1394322 | 45.837 | XSEC_3 |
| 13184 | 5222431 | 1394325 | 45.713 | XSEC_3 |
| 13185 | 5222431 | 1394325 | 45.854 | XSEC_3 |
| 13186 | 5222432 | 1394327 | 45.942 | XSEC_3 |
| 13187 | 5222434 | 1394332 | 45.802 | XSEC_3 |
| 13188 | 5222437 | 1394336 | 45.797 | XSEC_3 |
| 13189 | 5222439 | 1394341 | 45.75 | XSEC_3 |
| 13190 | 5222442 | 1394345 | 45.819 | XSEC_3 |
| 13191 | 5222445 | 1394349 | 46.013 | XSEC_3 |
| 13192 | 5222447 | 1394354 | 46.111 | XSEC_3 |
| 13193 | 5222452 | 1394370 | 46.271 | XSEC_3 |
| 13194 | 5222454 | 1394374 | 46.387 | XSEC_3 |
| 13195 | 5222457 | 1394379 | 46.456 | XSEC_3 |
| 13196 | 5222459 | 1394383 | 46.427 | XSEC_3 |
| 13197 | 5222461 | 1394388 | 46.7 | XSEC_3 |
| 13198 | 5222462 | 1394389 | 46.292 | XSEC_3 |
| 13199 | 5222464 | 1394393 | 46.061 | XSEC_3 |
| 13200 | 5222466 | 1394397 | 45.918 | XSEC_3 |
| 13201 | 5222466 | 1394397 | 46.003 | XSEC_3 |
| 13202 | 5222467 | 1394399 | 46.163 | XSEC_3 |
| 13203 | 5222468 | 1394402 | 46.073 | XSEC_3 |
| 13204 | 5222469 | 1394405 | 45.808 | XSEC_3 |
| 13205 | 5222470 | 1394407 | 45.844 | XSEC_3 |

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| 13206 | 5222473 | 1394411 | 45.934 | XSEC_3 |
| 13207 | 5222475 | 1394416 | 45.95 | XSEC_3 |
| 13208 | 5222477 | 1394421 | 46.083 | XSEC_3 |
| 13209 | 5222479 | 1394425 | 46.385 | XSEC_3 |
| 13210 | 5222481 | 1394430 | 46.529 | XSEC_3 |
| 13211 | 5222483 | 1394434 | 46.29 | XSEC_3 |
| 13212 | 5222483 | 1394434 | 46.666 | XSEC_3 |
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| 484 | 5221647 | 1394560 | 49.13 | XSEC_5 |

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| 12719 | 5221428 | 1395014 | 52.436 | XSEC_6 |

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| 395 | 5221197 | 1395148 | 53.607 | XSEC_7 |
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| 407 | 5221371 | 1395370 | 53.159 | XSEC_7 |
| 408 | 5221373 | 1395372 | 53.446 | XSEC_7 |
| 409 | 5221373 | 1395373 | 53.536 | XSEC_7 |
| 410 | 5221374 | 1395374 | 53.493 | XSEC_7 |
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| 328 | 5220624 | 1395577 | 57.563 | XSEC_9 |
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| 141 | 5219015 | 1396880 | 69.772 | XSEC_15 |

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| 10675 | 5213438 | 1398419 | 125.614 | XSEC_31 |

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